

# Ruddlesden geotechnical Itd

# Geotechnical Investigation and Contamination Assessment Report



Park Farm, Gillingham, Dorset

C.G. Fry & Son Ltd

March 2010

SR/SB/DT/09157/GICAR

# **REPORT CONTROL SHEET**

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	DORSET
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# **APPENDICES**

APPENDIX A EXPLORATORY HOLE RECORDS AND FIELD DATA

Trial Pit Logs (19 pages)

Soakaway Test Results (9 pages)

In-Situ CBR (TRL DCP Method) Test Results (8 pages)

APPENDIX B PHOTOGRAPHS (4 pages)

APPENDIX C LABORATORY TESTING RESULTS

Geotechnical Laboratory Testing (8 pages)

Contamination Laboratory Testing (13 pages)

APPENDIX D LANDMARK ENVIROCHECK REPORT (70 pages)

APPENDIX E SITE PLANS

Site Location Plan (1 page)

Exploratory Hole Location Plan (1 page)



# **EXECUTIVE SUMMARY**

It is proposed to develop land at Park Farm, Gillingham, Dorset, for residential purposes. Old maps showed the site has remained undeveloped from first edition maps (1887) until the present day, comprising open land since this time. The general field structure has remained largely the same throughout this period.

Nineteen trial pits typically encountered ground conditions of topsoil and/ or made ground, underlain by firm to stiff orangey-brown silty clay, underlain by bluish grey silty clay (occasionally with sand and gravel).

Foundation recommendations are traditional strip or trench-fill foundations at a minimum depth of 1.00m below existing or proposed ground levels, whichever is deepest, with deepening where building near trees in accordance with NHBC Standards for soils of high volume change potential.

A CBR value of 2% has been obtained from in-situ testing and this value may be used for road pavement design.

In-situ soakaway testing showed that the ground has a particularly low permeability and is not favourable for the adoption of soakaway drainage.

The contamination risk assessment indicates that the recorded levels of contamination do not pose a significant possibility of significant harm to human health or controlled waters and so no specific remedial measures are required for the proposed end-use.

No radon or ground gas protective measures are required.

As the trial pits were widely spaced to provide an initial understanding of the ground conditions only, more intensive trial pitting and testing is recommended prior to construction to confirm the above recommendations. In particular, the presence or absence of soft spots and the volume change potential should be determined in each area of the site.

# 1.0 <u>INTRODUCTION</u>

# 1.1 General

In January 2010, a combined Phase 1 and Phase 2: Geotechnical Investigation and Contamination Assessment was undertaken by Ruddlesden geotechnical ltd on behalf of C.G. Fry & Son Ltd, for the proposed residential development of land at Park Farm, Gillingham, Dorset.

The investigation was undertaken to determine subsurface ground conditions, to provide recommendations for foundations and associated structures, and to assess the extent of any contamination at the site.

The investigation comprised a desk study and walkover survey followed by the formation of nineteen trial pits with in-situ and laboratory testing.

# 1.2 Scope of Investigation

This investigation is intended to provide an overview of the ground conditions at the site and covers geotechnical and contamination aspects relating to the development. The brief was understood to comprise the following:

- Carry out a desk study and walkover survey.
- Undertake exploratory holes.
- Schedule geotechnical and contamination laboratory testing.
- Establish the ground conditions across the site.
- Make recommendations for foundation design.
- Carry out in-situ soakaway testing and provide recommendations for soakaway design.
- Make recommendations covering other geotechnical aspects, including roads, excavations and groundwater.
- Undertake a contamination risk assessment.
- Provide details of any contamination remedial measure requirements.



# 1.3 Scope of Report

The report is presented as a description of the procedures employed and the data obtained. This is followed by a thorough description of the ground and groundwater conditions, together with an assessment of material and mass ground parameters. The final part of the report comprises analysis, recommendations, and conclusions, which are provided in two separate parts: geotechnical and contamination.



### 2.0 THE SITE

### 2.1 **Site Location**

The site is located at Park Farm, Gillingham, Dorset, see Appendix E (Dwg. No. 09157/01). The British National Grid Reference of the site is 382320, 125930.

The site is located within a residential/ rural area to the southeast of the town of Gillingham. The surrounding topography is very gently undulating.

Access is gained via Carne Avenue and a padlocked set of gates in the west of the site or via Park Farm in the south.

### 2.2 **Site Description**

The site is irregular in shape, measuring approximately 930m x 700m (35Ha), and is gently undulating with an overall general gentle slope down towards the east-northeast. The ground was noted to be boggy underfoot across much of the site.

The site comprises eight grass-covered fields, divided by hedgerows and drainage ditches. Ponds are present in the north and northwest of the site, and an attenuation pond is present in the east of the site. Enclosed areas of relatively recently planted mixed deciduous trees, no taller than approximately 15m, are present in the north, east and southwest of the site. A small field, currently in use as an allotment, with hutches and pens, is present in the southwest of the site. Mixed deciduous trees, no taller than approximately 20m, typically surround the fields.

Anecdotal information from the current owner of the site indicates that the fields have largely been used for grazing and that the field in the south/ southeast of the site has recently been re-profiled with a coverage of approximately 1m deep made ground (clay subsoil).

The site is bordered to the north by a brook (Fern Brook) before fields, to the east by a stream before fields, to the south by a working farm (Park Farm) and to the west by a housing estate, a business park (Kingsmead Business Park) and an electricity housing substation.

Photographs of the site are presented in Appendix B.



### 3.0 **DESK STUDY**

### 3.1 General

A desk study was undertaken to provide background information, comprising the consultation of:

- Old Ordnance Survey maps.
- Geological maps and memoirs.
- Environmental information.

This information was used to produce a "conceptual model" of the site so that an appropriate intrusive investigation could be carried out.

### 3.2 **Site History**

A full set of old Ordnance Survey maps of the site was obtained as part of the Landmark Envirocheck report (Appendix D of this report). The salient points are described below.

In summary, the old Ordnance Survey maps showed that the site has remained undeveloped from first edition maps (1887) until the present day, comprising open land since this time. The general field structure has remained largely the same throughout this period.

### 3.3 **Site Geology**

The British Geological Survey (BGS) map of the area shows the site to be underlain by Jurassic Kimmeridge Clay Formation, overlain by Quaternary Head deposits in the north of the site.

The BGS regional guide describes the Kimmeridge Clay Formation as "Mudstones; thin siltstone and cementstone beds; locally sands and silts". The Head deposits are described as "silty to sandy clay with angular clasts of local rock fragments up to boulder size".

### 3.4 **Environmental Information**

The environmental information contained within the Landmark *Envirocheck* Report is listed below:

- The nearest surface water feature is located on site, comprising a pond in the northwestern part of the site and an attenuation pond and stream (Fern Brook) in the eastern part of the site.
- There are no known abstraction points within 250m of the site.
- The underlying strata are classified as a Non Aquifer in the southwest and a Minor Aquifer in the north and east. Non Aquifers are generally regarded containing insignificant quantities of groundwater. groundwater flow through such rocks, although imperceptible, does take place and needs to be considered in assessing the risk associated with persistent pollutants. Minor Aquifers can be fractured or potentially fractured rocks, which do not have a high primary permeability, or other formations of variable permeability including unconsolidated deposits. Although not producing large quantities of water for abstraction, they are important for local supplies and in supplying base flow to rivers.
- There are no recorded landfill sites within 250m of the site.
- The British Geological Survey (BGS) information indicates that between 1% and 3% of homes are above the Radon Action Level but that no radon protective measures are necessary in the construction of new dwellings.

### 3.5 **Initial Conceptual Model**

# 3.5.1 Geotechnical Conceptual Model

From the historical data, the site has not had an industrial past-use. However, anecdotal evidence and discussions with the current land owner suggest that part of the site has been re-profiled and so some made ground would be expected in parts of the site.



From the published information, the expected underlying geology is Kimmeridge Clay Formation, possibly overlain by Quaternary Head deposits in the north of the site. The Kimmeridge Clay deposits should provide sufficient bearing capacity for the adoption of traditional strip or trench-fill foundations, though the Head deposits may be too weak.

3.5.2 Contamination Conceptual Model

Source

Old maps showed that the site has comprised a series of fields since 1887. Although such a land-use is unlikely to have caused any significant contamination of the ground, some made ground may be present and this

may be generically contaminated.

Located in a predominantly rural area, and based on desk study information, it is considered that past and present surrounding land uses are unlikely to have caused any significant contamination of the ground beneath the site.

**Pathway** 

In accordance with the CLEA model for a residential land use, exposure pathways potentially linking contamination to humans include:

Direct soil and indoor dust ingestion.

Consumption of homegrown produce.

Consumption of soil adhering to homegrown produce.

· Skin contact with soils and indoor dust.

• Inhalation of indoor and outdoor dust and vapours.

If present, groundwater flow is considered to be the main migration pathway linking any contamination to controlled waters receptors.

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Receptor

As a residential land use, end-users are considered as potential receptors of

any contamination, with a young female child (aged zero to six years old),

being the critical receptor.

The nearest water course, located on site, and groundwater beneath the site

are considered to be the main potential controlled waters receptors.

3.6 **Sampling and Analysis Plan** 

In order to confirm the above conceptual models, an intrusive site

investigation was undertaken.

The purpose of this investigation is to identify any areas of the site where

abnormal subsurface costs might exist, e.g. contamination, piled foundations

etc. Additional, more intensive, investigation work is likely to be required in the

future. Nevertheless, it is intended that this investigation will provide

parameters to enable initial foundation, road and drainage design to be

carried out for costing purposes and identify any probable contamination

remediation costs.

Trial pits were considered to be the most suitable exploratory technique, as

these would enable a large volume of the ground to be inspected and tested

in-situ. The trial pits were located so as to provide a reasonable spread of

information and an accurate representation of subsurface ground conditions.

In-situ Dynamic Cone Penetrometer (DCP) testing was undertaken to provide

an indication of California Bearing Ratio (CBR) value for use in road pavement

design.

In-situ soakaway testing was undertaken to assess the permeability and

suitability of the ground for soakaway drainage.

Geotechnical laboratory tests were undertaken to determine the volume change potential of the soil for foundation design and pH and sulphate testing to determine concrete class requirements. Samples were taken for geotechnical testing from seventeen different locations, at different depths.

Representative samples were tested for the former ICRCL suite of contaminants, speciated PAH and total TPH, which provides a broad and general suite of contaminants that may be present. Samples were selected for contamination testing from eighteen of the trial pits from a range of depths within the near surface deposits, as, in accordance with the CLEA model, contamination is assumed to be within the near surface deposits for most exposure pathways.

Samples were selected for testing to provide an accurate representation of ground conditions encountered.



### 4.0 **FIELDWORK**

### 4.1 General

All fieldwork was undertaken on 25 and 26 January 2010. The siting and setting out of all the trial pits was the responsibility of Ruddlesden geotechnical ltd, who also determined the extent of testing and sampling.

The number of trial pits was limited to minimise disturbance to the farmer's fields, but were sufficient to broadly understand the ground conditions.

All fieldwork was undertaken in accordance with BS5930 (1999): British Standard Code of Practice for Site Investigation, British Standard BS10175 (2001): Investigation of Potentially Contaminated Sites – Code of Practice and Eurocode 7 (2007): Part 2 Ground Investigation and Testing.

### 4.2 **Trial Pits**

Nineteen trial pits were excavated to depths of between 2.60m and 3.00m using a JCB 8080 (eight tonne tracked excavator).

Samples and observations were made from inside the pit to a depth of 1.20m, where safe to do so, from the surface and from samples recovered from the excavator bucket. The supervising geologist provided a detailed description of the ground conditions, groundwater and stability and also obtained samples at representative locations, which were placed into suitable containers. The trial pits were not shored.

In-situ shear vane testing was undertaken in suitable soils to obtain an estimate of undrained shear strength.

Details of ground and groundwater conditions encountered can be found on the trial pit logs (Appendix A) and photographs (Appendix B). The trial pit locations are shown on the exploratory hole location plan (Dwg. No. 09157/02, Appendix E).

# 4.3 Soakaway Testing

Three soakaway tests were undertaken in general accordance with BRE 365 "Soakaway Design".

The trial pit was excavated to a depth deemed sufficient to represent a section of the design soakaway. The vertical sides were trimmed square. A 1500-gallon water bowser was used to supply the large volumes of water required at a quick rate.

The pit was filled with water and allowed to drain. The fall in water level was recorded with time.

# 4.4 In-Situ CBR (TRL DCP Method) Testing

In-situ Dynamic Cone Penetrometer (DCP) Testing was undertaken at eight locations across the site, as shown on the exploratory hole location plan (Dwg. No. 09157/02, Appendix E).

The Transport Research Laboratory (TRL) DCP uses an 8kg hammer dropping through a height of 575mm and a 60° cone having a maximum diameter of 20mm. The penetration and number of blows are recorded up to a maximum depth of 1.00m BGL. The penetration rate is recorded as the cone is driven into the subgrade and is used to calculate the strength of the material (CBR value) through which the cone is passing. A change in penetration rate indicates a change in strength between materials, thus allowing layers to be identified and the thickness and strength of each to be determined.



# 5.0 LABORATORY TESTING

# 5.1 **General**

All laboratory testing was scheduled by Ruddlesden geotechnical ltd and the results are presented in Appendix C of this report.

# 5.2 **Geotechnical Testing**

The programme of laboratory testing was carried out in accordance with BS 1377 (1990) "Methods of Test for Soils for Civil Engineering Purposes".

The following tests were carried out on seventeen samples:

- Moisture Content
- Plasticity Tests
- Particle Size Distribution
- pH Value
- Sulphate Content

# 5.3 Contamination Testing

In order to test the conceptual model of the site (see section 3.5.2 of this report), eighteen soil samples were tested for the following suites of tests; the testing was UKAS accredited:

# **Former ICRCL Suite**

Arsenic, cadmium, chromium, copper, lead, mercury, nickel, selenium, zinc, total PAH, total sulphate, soluble sulphate, pH, boron, phenols.

# **Speciated Polyaromatic Hydrocarbons (PAH)**

Acenaphthene, acenaphthylene, anthracene, benzo(a)anthracene, benzo(a)pyrene, benzo(b)fluoranthene, benzo(g,h,i)perylene, benzo(k)fluoranthene, chrysene, dibenzo(a,h) anthracene, fluoranthene, fluorene, indeno(1,2,3-cd)pyrene, naphthalene, phenanthrene, pyrene.

# **Total Petroleum Hydrocarbons (TPH)**

Total TPH.

# **Soil Organic Matter (SOM)**



### 6.0 **RESULTS OF THE INVESTIGATION**

### 6.1 General

The following sections provide a summary of ground conditions encountered, groundwater and laboratory testing. Further details are provided in the Appendices of this report.

The results of this investigation broadly concur with the predicted conceptual model.

### 6.2 **Ground Conditions Encountered**

# 6.2.1 Topsoil

Topsoil was encountered in all of the trial pits to depths of between 0.20m and 0.40m below existing ground levels.

# 6.2.2 Made Ground

Made ground was encountered in TP18 and TP17 beneath the topsoil, to a depth of 1.00m and 1.20m respectively.

In TP17, the made ground was observed to comprise orangey brown silty gravelly clay, to a depth of 1.00m, underlain by soft orangey brown silty slightly gravelly clay to a depth of 1.20m. In TP18, the made ground was observed to comprise orangey brown silty slightly gravelly clay with occasional pieces of brickwork and ceramics to a depth of 1.00m



6.2.3 Natural Geology

Beneath the topsoil and/ or made ground, firm to stiff orangey-brown

occasionally mottled bluish grey silty clay was typically encountered to depths

of between 0.70m and 2.80m.

In TP02 and TP09, the clay was observed to be slightly gravelly at depths of

0.60m and 1.30m, respectively. In TP01, TP05 and TP08, the clay was

observed to be gravelly at depths of 0.60m, 1.30m and 1.50m, respectively. In

TP16, the clay was observed to be very gravelly at 0.90m.

In TP10 and TP13, this clay deposit was underlain by orangey brown or bluish

grey medium dense clayey (TP10) slightly sandy gravel to a depth of 2.20m.

In all the trial pits these deposits were typically underlain by firm to stiff bluish

grey, occasionally mottled orange or brown, silty clay, to the base of all the

trial pits (up to 3.00m).

In TP01, this deposit was recorded as being slightly silty with occasional

pockets of gravel. Frequent pieces of shell material were also observed in

both TP01 and TP05.

In TP03 and TP07, it was intersected by a layer of medium dense bluish grey

slightly clayey silty (TP03) gravelly sand at depths of between 2.00m to 2.40m

and 2.30m to 2.70m, respectively.

In TP04, TP06, TP09, TP11 and TP18, the clay was observed to become

sandy and/ or gravelly at depths of 2.40m, 2.80m, 1.90m, 2.70m and 2.30m

respectively.

Estimates of undrained shear strength obtained from in-situ shear vane

testing at a depth of 1.00m typically ranged from 70kN/m2 to 130kN/m2,

indicating the ground to be firm to stiff. At one location (TP03) a shear

strength value of 40kN/m<sup>2</sup> was recorded, indicating the ground in this location to be soft to firm, at this depth.

The density of the granular deposits was estimated from a visual assessment only, i.e. ease of excavation and stability of trial pit sides.

# 6.3 **Groundwater**

Groundwater was encountered at the following depths during the course of the investigation:

Table One: Occurrence of Groundwater During Excavation (25/01/10)

BH No.	Water Level (mBGL)	Rate of Inflow
TP03	2.20	Slight
TP04	2.50	Slight
TP07	2.70	Slight

# 6.4 Soakaway Testing

Full details of the soakaway testing results are provided in Appendix A of this report and are summarised in the table below:

**Table Two: Summary of Soakaway Test Results** 

Test No.	Total Recorded  Fall of Water  Level (m)  Duration of Test  (minutes)		Soil Infiltration Rate (m/s)	
TP02	-0.02	420	* N/A	
TP09	-0.33	372	* N/A	
TP15	-0.01	308	* N/A	

<sup>\*</sup> Rise in water level recorded. No calculation of soil infiltration rate possible. Tests are deemed to have failed.



# 6.5 <u>In-Situ CBR Testing (TRL DCP Method)</u>

The results of the in-situ dynamic cone penetrometer (DCP) testing are presented in Appendix A of this report.

From the DCP testing, CBR values ranging from 2% to 43% have been obtained.

# 6.6 Geotechnical Laboratory Testing

All the geotechnical laboratory testing results are presented in Appendix C of this report and are summarised in the table below:

**Table Three: Summary of Geotechnical Laboratory Testing Results** 

	TP01	TP03	TP04	TP05	TP06	TP07	TP08	TP10	TP11
	1.00m	3.00m	2.50m	1.00m	2.00m	1.50m	2.00m	1.00m	2.00m
Moisture Content (%)	27.7	35.6	24.4	22.8	27.6	26.6	24.4	12.6	25.4
Liquid Limit (%)	62	74	-	52	49	51	58	-	53
Plastic Limit (%)	23	27	-	19	18	18	23	-	19
Plasticity Index (%)	39	47	-	33	31	33	35	-	34
%passing 425µm sieve	100	100	43	100	100	100	100	32	100
Modified Plasticity Index (%)	39	47	-	33	31	33	35	-	34
Volume Change Potential	Medium	High	Non- shrinkable	Medium	Medium	Medium	Medium	Non- shrinkable	Medium
pH Value	7.6	6.4	7.8	7.6	7.9	8.1	7.8	8.0	8.0
Sulphate Content (g/l)	0.08	0.01	0.02	0.01	0.01	0.01	0.01	0.07	0.01



	TP12	TP13	TP14	TP15	TP16	TP17	TP18	TP19
	1.50m	2.00m	1.50m	1.00m	1.25m	2.50m	1.00m	1.00m
Moisture Content (%)	29	21.2	22.7	28.2	21	27.5	19.6	25.7
Liquid Limit (%)	49	-	63	65	-	60	74	57
Plastic Limit (%)	17	ı	19	21	-	21	25	21
Plasticity Index (%)	32	-	44	44	-	39	49	36
%passing 425µm sieve	100	27	100	100	51	100	100	100
Modified Plasticity Index (%)	32	-	44	44	-	39	49	36
Volume Change Potential	Medium	Non- shrinkable	High	High	Non- shrinkable	Medium	High	Medium
pH Value	8.0	7.9	5.6	8.0	7.8	8.0	7.9	8.2
Sulphate Content (g/l)	0.24	0.03	0.01	0.1	0.03	0.22	0.13	0.05

### 6.7 **Contamination Laboratory Testing**

All the laboratory testing results, together with the Generic Assessment Criteria to which they have been compared, are presented in Appendix C of this report and the implications are discussed in section 8 of this report.

In summary, no significantly elevated levels of contamination were recorded in any of the eighteen samples tested.



### 7.0 **GEOTECHNICAL ASSESSMENT**

### 7.1 **Proposals**

It is understood that the site is to be developed for residential purposes with the construction of several houses with associated infrastructure. No further details were available at the time of writing this report.

### 7.2 **Ground Profile**

The ground conditions encountered have been summarised in section 6.0 of this report and the individual trial pit logs, photographs and laboratory testing results should be referred to for further details. Within this section of the report the general ground profile is reviewed and the engineering significance of individual layers is discussed.

Beneath a surface covering of topsoil and/ or made ground, firm to stiff orangey-brown becoming bluish grey silty clay (occasionally with varying proportions of sand and gravel) was typically encountered.

Laboratory testing revealed the clay to be of medium to high volume change potential in accordance with NHBC Standards, chapter 4.2. In the absence of intensive testing, high volume change potential soils should be assumed.

Estimates of undrained shear strength obtained from in-situ shear vane testing at a depth of 1.00m typically ranged from 70kN/m2 to 130kN/m2, indicating the ground to generally have sufficient bearing capacity to provide a suitable founding stratum for strip or trench-fill foundations.

However, as an exception, at one location (TP03) a shear strength value of 40kN/m<sup>2</sup> was recorded, indicating the foundations in this area would require deepening or reinforcing. This might also indicate that other similar areas are present elsewhere across the site.



### 7.3 **Foundations**

# 7.3.1 General

The results of this investigation indicate that strip or trench-fill foundations are generally suitable to support the proposed structures.

However, prior to development, further, more intensive, testing should be carried out to confirm that ground of sufficient bearing capacity is present and to confirm the volume change potential.

# 7.3.2 Strip or Trench-Fill Foundations

It is considered that a safe nett allowable bearing pressure of 150kN/m<sup>2</sup> may be placed on the firm to stiff clay by strip or trench-fill foundations of least width 600mm at a minimum depth of 1.00m below existing or proposed ground levels, whichever is deepest.

Where building near trees, foundations should be deepened in accordance with NHBC Standards, chapter 4.2, for soils of high volume change potential. However, more intensive testing might prove high volume change potential soils to locally be absent. Foundations may, therefore, in some areas of the site, be able to be deepened for soils of medium volume change potential.

Where foundations are stepped to take account of the influence of trees they should be stepped gradually with no step exceeding 0.50m.

Heave precautions are required to protect the foundations from lateral soil heave movements where the foundation is within the influence of trees and where the foundation depth is greater than 1.50m due to NHBC requirements where building near trees. Suitable heave precautions for trench-fill foundations would be compressible material against the inside faces of all external wall foundations.



It should be endeavoured to build foundations on like material. Where this is not possible, the boundary between the changes in strata, e.g. clay/ gravel, should be suitably reinforced.

Foundations must also be built at least 0.20m below any made ground.

Any soft or loose material in the base of foundation excavations should be removed and replaced with compacted lean mix concrete prior to pouring the foundations.

# 7.3.3 Ground Floor Slabs

Where more than 600mm of made ground is present, where the slope of the ground means that more than 600mm of fill is required beneath floor slabs or where NHBC building near trees requirements mean that foundation depths are greater than 1.50m, fully suspended ground floor slabs are required.

Where less than 600mm of made is present, where the slope of the ground means that less than 600mm of fill is required beneath floor slabs and where NHBC building near trees requirements mean that foundation depths are less than 1.50m, ground bearing slabs may be adopted.

As the soils are of medium to high volume change potential, suspended ground floor slabs should be used where ground floor construction is undertaken when soils are seasonally desiccated (i.e. during summer months and autumn).

# 7.3.4 Sulphate and pH Aggressivity

The results of the pH and sulphate tests have been compared to Table C1 of BRE Special Digest 1 "Concrete in Aggressive Ground". This comparison indicates the Design Sulphate Class for the site to be DS- 1. As the site is considered to be greenfield, groundwater can be treated as static and pH values greater than 2.5 were recorded, Aggressive Chemical Environment for

Concrete (ACEC) class AC- 1s is required for all buried concrete at this site,

i.e. no special precautions.

7.3.5 Radon Protective Measures

BR Report 211 "Radon: Guidance on Protective Measures for New Dwellings"

and British Geological Survey (BGS) information obtained as part of the

Landmark Envirocheck report (Appendix D) indicate that no radon protection

measures are required.

7.4 **Groundwater and Excavations** 

Slight groundwater seepage was encountered in TP03, TP04 and TP07 at

depths of between 2.20m and 2.70m. No groundwater was encountered in

any of the other trial pits. Some de-watering of deep excavations is therefore

likely to be required.

It is noted that groundwater levels fluctuate according to the season and from

year to year. It is noted that in the weeks prior to the investigation the weather

had been wet for the time of year. Therefore, lower groundwater levels may

be encountered during the drier summer months. Likewise though, higher

groundwater levels may be encountered during periods of wetter weather.

All trial pits were generally stable and so only limited shoring of temporary

excavations should be necessary.

No problems with excavatability are foreseen.

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7.5 Roads

In-situ CBR (TRL DCP Method) testing produced CBR values ranging from

2% to 43%.

The TRL DCP can sometimes produce artificially high CBR values. It is

therefore recommended the lower CBR value of 2% be used for road

pavement design at this site.

This should be confirmed prior to construction with full-scale in-situ CBR tests

at road level in accordance with BS1377 and to the satisfaction of the

adopting authority.

Laboratory testing indicated the soils to be frost-susceptible.

7.6 **Soakaways** 

Three in-situ soakaway tests were undertaken in accordance with BRE 365.

Water level rises of 0.01m, 0.02m and 0.33m were recorded over the course

of the day.

These results indicate that the ground has a very low permeability and is not

suitable for the adoption of soakaway drainage.

The preferable drainage solution at this site would be to discharge into the

sewer. If necessary, an underground storage tank with a throttled outflow

valve may be able to be installed to allow water to be discharged into the

sewer at an agreed rate with the local water authority so that during storm

periods discharge into the sewer is not increased from the present situation.

The use of Attenuation Cells, (also known as Storm Cells, Soakaway

Modules, and Storm Crates) might be appropriate at this site. These are

specially manufactured, modular cells usually made from recycled

polypropylene that are light in weight. Being modular, many units can be linked together, in three planes, to give whatever capacity is required.

Surface water run-off volumes may also be reduced by the adoption of permeable paving and/ or the adoption of rainwater harvesting/ recycling techniques.

# 7.7 Further Work

The spacing of the trial pits was necessarily relatively far apart, to minimise disturbance to the farmer's fields, but at the same time were sufficiently close together to enable subsurface ground conditions to be relatively well understood so that preliminary subsurface costs (foundations, drainage, road design, contamination etc) could be ascertained.

Prior to development, more intensive trial pitting and testing is recommended to confirm the findings of this report. In particular, slightly softer ground was encountered in TP03; the precise extent of this is currently unknown and similar soft areas might exist elsewhere across the site. Also, laboratory testing classified most of the soils as either high or medium volume change potential. In the absence of further testing, foundations must be designed in accordance with NHBC Standards, Chapter 4.2 for high volume change potential soils; intensive testing might prove that some areas can be classified as medium volume change potential, allowing foundation depths and heave protection measures to be reduced.



### 8.0 **CONTAMINATION ASSESSMENT**

### 8.1 General

It is understood that the site investigated is to be developed for residential purposes with houses and flats, with private and communal gardens and parking areas. No further details were available at the time of writing this report.

The contamination assessment has been carried out in accordance with the latest Environment Agency guidance using a source-pathway-receptor analysis method, so that appropriate remedial measures may be proposed.

### 8.2 **Human Health Risk Assessment**

### 8.2.1 **Generic Assessment Criteria**

A Generic Qualitative Risk Assessment (GQRA) has been undertaken to assess the level of risk posed to human health by soil contamination.

The results of the contamination laboratory testing have been compared to Generic Assessment Criteria (GAC) to aid the evaluation of the extent of contamination at the site. If any of the GAC are exceeded, this may be indicative of an unacceptable risk to the health of site-users and that further investigation and/ or remediation is required.

The proposed end-use of residential land use has been used in this risk assessment.

Where Soil Guideline Values (SGV's), published by DEFRA and derived from the Contaminated Land Exposure Assessment (CLEA) model, are available, the results of the laboratory testing have been compared against the published SGV's for the proposed end-use.

For analytes where SGV's have not yet been produced, GAC produced by Management (LQM) and the Chartered Institute of Land Quality



Environmental Health (CIEH) have been referenced. The LQM/ CIEH GAC have been derived using the DEFRA and Environment Agency CLEA UK (1.04) model, which is the same methodology as the Government's Soil Guideline Values (SGV's) and is the Environment Agency's currently recommended exposure model.

In the absence of a SGV or LQM/ CIEH GAC, for determinands that are either not particularly harmful to human health or for which toxological and physiochemical information is particularly difficult to obtain, the Dutch or ICRCL intervention values have been used as initial screening values. A Detailed Quantitative Risk Assessment (DQRA) is undertaken if any of these initial screening values are exceeded.

For determinands that are primarily deleterious to building materials, levels provided in BRE Special Digest 1, Concrete in Aggressive Ground, are considered to be the most appropriate for comparison.

# 8.2.2 Comparison of Testing Results to GAC

Of the eighteen soil samples tested, the following Generic Assessment Criteria were exceeded for a residential land use:

**Table Four: Contamination Testing Results Exceeding GAC** 

Determinand	Unit	GAC	Highest Recorded Value	Location of Highest Recorded Value	No. of values exceeding GAC	Source of GAC
Arsenic	mg/kg	32	72	TP13	1 of 18	SGV
Nickel	mg/kg	130	180	TP13	1 of 18	SGV

Although elevated levels of arsenic and nickel were recorded in TP13 (72mg/kg and 180mg/kg respectively), a statistical test (mean value test) was undertaken on all arsenic and nickel results. This statistical analysis indicates that the upper bound values (US<sub>95</sub>) to be 20.48mg/kg for arsenic and 57.84mg/kg for nickel, which are less than the respective Soil Guideline

Values of 32mg/kg and 130mg/kg. Therefore, the recorded levels of arsenic and nickel are not considered to be significant.

### 8.3 **Controlled Waters Risk Assessment**

In order for land affected by contamination to cause harm, there must be a source of contamination, a receptor that can be harmed and a pathway by which the receptor can be exposed to the contamination.

As no significantly elevated levels of contamination were recorded (i.e. there is no source) and no significant groundwater was encountered (i.e. there is no pathway), it is considered that there is no significant possibility of significant harm to controlled waters.

### 8.4 **Ground Gas Assessment**

The breakdown of organic material in made ground can produce ground gas, though it may also be produced by other, natural, sources (e.g. coal, peat). The principal components of ground gas are methane (potentially explosive) and carbon dioxide (potential asphyxiant).

There are no recorded landfill sites within 250m of the site and made ground was encountered at relatively shallow depths in only two of the trial pits.

Therefore, ground gas protection measures are not considered to be required at this site.

### 8.5 **Revised Conceptual Model**

Prior to the investigation, it was considered unlikely that any significant contamination would be present.

The results of this investigation indicate no significantly elevated levels of contamination to be present at this site.

Park Farm, Gillingham

Ruddlesden geotechnical ltd

Therefore, the "source" is not present in the source-pathway-receptor chain.

**Discussion and Recommendations** 8.6

From the results of the contamination risk assessments, due to the absence of

a contamination source, it is considered that the levels of contamination

recorded in this investigation do not present a significant possibility of causing

significant harm to human health or controlled waters and so no specific

remedial measures are required for the proposed end-use.

However, if any unexpected discoveries are encountered during construction

activities (i.e. anything substantially different from the findings of this

investigation), Ruddlesden geotechnical ltd should be contacted so that

appropriate recommendations may be provided.

Also, in line with general good practice, comprehensive and accurate site

records should be kept, including details of where soil has been moved to or

from site and tip receipts.

If contamination aspects are a planning condition, these recommendations are

subject to the approval of the local authority.

8.7 Off-Site Disposal of Excavated Soils

The Waste Acceptance Criteria (WAC) testing results indicate that excavated

soil is likely to be classified as Inert Waste for off-site disposal purposes.

However, this classification should be confirmed by passing these results to a

licensed tip operator.

If necessary and required by the tip operator, Waste Acceptance Criteria

(WAC) testing could be carried out on soil to be removed from site to confirm

the classification of the soil.

### 9.0 **CONCLUSIONS**

- 1. The site has remained undeveloped from first edition maps (1887) until the present day, comprising open land since this time. The general field structure has remained largely the same throughout this period.
- 2. Ground conditions encountered were typically topsoil and/ or made ground, underlain by firm to stiff orangey-brown silt clay, underlain by bluish grey silty clay (occasionally with varying proportions of sand and gravel).
- 3. Strip or trench-fill foundations are recommended at a minimum depth of 1.00m below existing or proposed ground levels, whichever is deepest, with deepening where building near trees in accordance with NHBC Standards for soils of high volume change potential.
- 4. In-situ testing indicated that a CBR value of 2% may be used for road pavement design.
- 5. The results of the in-situ soakaway testing indicate that the ground has a particularly low permeability and is not favourable for the adoption of soakaway drainage.
- 6. The contamination risk assessment indicates that the recorded levels of contamination do not pose a significant possibility of significant harm to human health or controlled waters and so no specific remedial measures are required for the proposed end-use.
- 7. No radon or ground gas protective measures are required.
- 8. As the trial pits were widely spaced to provide an initial understanding of the ground conditions only, more intensive trial pitting and testing is confirm recommended prior to construction to the above recommendations. In particular, the presence or absence of soft spots and the volume change potential should be determined in each area of the site.

### 10.0 <u>REFERENCES</u>

- British Geological Survey (1996): England and Wales Sheet 297, Wincanton.
- British Standard BS5930 (1999): Code of Practice for Site Investigation.
- British Standard BS10175 (2001): Investigation of Potentially Contaminated Sites – Code of Practice.
- Building Research Establishment (2001): Special Digest 1: Concrete in Aggressive Ground.
- Building Research Establishment (2007): Report BR 211: Radon: Guidance on Protective Measures for New Dwellings.
- DEFRA & Environment Agency (2004): CLR 11: Model Procedures for the Management of Land Contamination.
- Environment Agency (2009): Human Health Toxological Assessment of Contaminants in Soil.
- Environment Agency (2009): Updated Technical background to the CLEA Model.
- Eurocode 7 (2007): Part 2 Ground Investigation and Testing.



### 11.0 **TERMS AND CONDITIONS**

- 1. This report has been prepared for the sole use of the specified client in response to an agreed brief and for the stated purpose. The recommendations used in this report should not be used for any other schemes on or adjacent to this site without further reference to this company.
- 2. The copyright of this report is owned by Ruddlesden geotechnical ltd. With the exception of the named client, who may copy and distribute the report to deal with matters directly relating to its commission, this report may not be reproduced, published or adapted without written consent of the company.
- 3. New information, improved practices and legislation may necessitate an alteration to the report in whole or in part after its submission. Therefore, with any change in circumstances, this report should be referred to Ruddlesden geotechnical ltd for reassessment and, if necessary, reappraisal.
- 4. The comments given in this report assume that ground conditions do not vary beyond the range revealed by the investigation. There may, however, be conditions at or adjacent to the site that have not been disclosed by the investigation and which, therefore, have not been considered in this report. Accordingly, a careful watch should be maintained during any future groundworks and the recommendations of this report reviewed as necessary.
- 5. Whilst confident in the findings of the report, the recommendations may not necessarily be accepted by other authorities without question. It is advisable that, where appropriate, the report be submitted to the relevant statutory authorities and approval obtained before detailed design, site works or other irrevocable action is undertaken.
- 6. All comments and recommendations are based on groundwater conditions encountered at the time of investigation. It should be noted that groundwater levels might fluctuate according to the season and from year to year. This may have implications on other recommendations, including foundations and excavations.

### **APPENDICES**



# APPENDIX A EXPLORATORY HOLE RECORDS AND FIELD DATA



#### **KEY TO TRIAL PIT AND BOREHOLE LOGS (COMMON SYMBOLS)**

#### STRATA LEGEND

Made Ground

**Topsoil** 

Clay

Silt

Sand

Gravel

Peat

Composite soil types will be signified by combined symbols, e.g. silty sand Chalk

Limestone

Coal

Mudstone

Siltstone

Sandstone

Fine grained igneous rock

(e.g. basalt)

Medium grained igneous

rock (e.g. granite)

Fine grained metamorphic

rock (e.g. slate)

#### **GROUNDWATER**



Groundwater strike



Standing groundwater level

#### INSTALLATIONS



**CEMENT SEAL** 

**BENTONITE SEAL** 

FILTER PACK (SLOTTED PIPE)

#### **SAMPLES**

D Small disturbed sampleJ Small disturbed sample

(amber glass jar)

B Disturbed bulk sample

U100 Undisturbed sample (100mm

diameter)

W Water sample

#### **IN-SITU TESTING**

SPT Standard Penetration Test (split spoon sampler)

SPT(C) Standard Penetration Test

(solid cone)

V Shear vane test

CBR California Bearing Ratio

(estimated from soil

assessment (mexe) cone

penetrometer)

#### **ROTARY DRILLING**

TCR Total core recovery (%)
SCR Solid core recovery (%)
RQD Rock quality designation (%)
FI Fracture Index (fractures/ m)

NI Non-intact

#### **SPT RESULTS (EXAMPLES)**

30 "N" Value (blows recorded for 300mm penetration, following 150mm seating

drive)

50/125 50 blows for 125mm

penetration



IDENTIFICATION AND DESCRIPTION OF SOILS (Taken from BS 5930: 1999, Table 13)

Term					IDENTIFICATION AND L		<u> </u>	11011 0	OOILO	/ (Takell II	0111 DO 393	0. 1333, 10	abic 13)				
		/S	rength	Disc	ontinuities	Ве	dding	Colour	Types (ı	mixtures o	T I		SOIL			Stratum	Example Descriptions
Red-rive with SPT Number   Very Box   Very	ry rse ils		By inspection of			Scale of bed	ding thickness					200	BOULDERS	complete in pits or			
Butterlook uith SPT N-value   Very   Over	Ve coa so	Dense		Term	spacing	Term	thickness		Term	Approx % <sup>c)</sup> secondary			COBBLES	recover whole from	pockets of peat,		sandy sub-angular
Model		Borehole with S	SPT N-value				Over 2000		Slightly	Æ		Coarse			flint gravel,	Alluvium	GRAVEL with small pockets (up to 30
Compared		Very loose	0-4	Widely			2000 to 600	Brown	(sandy <sup>d)</sup> )	73	rounded	Medium	GRAVEL	be described;			
Signify cerements of the removes present and the remov	(sez	Loose	4-10	Medium		bedded	600 to 200	Blue	(sandv <sup>d)</sup> )	5 to 20 b)		Fine			using terms such		
Sightly commend or maintaince, pick-moves processed from post commend which can be advanted to a be advanted	velsiz	Medium dense	10-30		200 to 60	bedded	200 to 60	Cream	(oana) /	0 10 20		_				Lias Clay	Medium dense light
Sightly commend or maintaince, pick-moves processed from post commend which can be advanted to a be advanted	<b>soils</b> nd gra	Dense	30-50	closely		bedded	60 to 20	Black		>20 <sup>b)</sup>		Coarse			with rare	Embankment	clayey fine SAND.
Signify cerements of the removes present and the remov	<b>irse</b> and ar	Very dense	>50		Under 20	laminated	20 to 6	etc.	(sandy <sup>9</sup> )		Elongated	0.6		Visible to naked	with occasional		
Signify cerements of the removes present and the remov	Coa 5% se						Under 6					Medium	SAND	No cohesion when	frequent/	Topsoil	
Compact   Comp			examination: pick removes soil in lumps which can be	Fissured	blocks along unpolished				AND	About 50 <sup>b)</sup>	constituent type  Calcareous,	0.2		Grading can be	% defined on a	or Glacial	sheared orange mottled brown slightly gravelly CLAY. Gravel is fine and medium of rounded
Compact Compac		Un-compact	or crushed in the	Sheared	blocks along polished	bedded	different types, prequalified by thickness term if in equal proportions. Otherwise thickness of		Term		Glauconitic, Micaceous  Using terms such as: slightly	Coarse 0.02	SILT	visible with hand lens; Exhibits little plasticity and marked dilatancy;	specific basis or		(Reworked Weathered London Clay)
Film   Fibre already   Compressed together   Fibrous	ay sizes)	Compact	or crushed by strong pressure			Inter-	between subordinate	Mottled		<35	calcareous	Fine		silky to the touch; Disintegrates in water;			laminated grey CLAY with closely spaced thick laminae of sand.
Film   Fibre already   Compressed together   Fibrous	ne soils silt and cl	C <sub>u</sub> 0 – 20kPa	pushed in up to 25mm	-		laminated					% defined on	3.002		broken but not powdered between			(Alluvium)
Counting the production of t	<b>Fi</b> (over ~35%	$\begin{aligned} &C_u\ 20-40 \text{kPa} \\ &\text{Firm} \\ &C_u\ 40-75 \text{kPa} \\ &\text{Stiff} \\ &C_u\ 75-150 \text{kPa} \end{aligned}$	up to 10mm  Thumb makes impression easily  Can be indented slightly by thumb	for distar partings, or lamina	nce between isolated beds ae, desiccation,				(sandy <sup>e)</sup> )	35 to 65 <sup>a)</sup>	material specific basis		CLAY	They also disintegrate under water but more slowly than silt; Smooth to the touch; Exhibits plasticity			clayey amorphous PEAT.
Firm Fibre already compressed together Fibrous Fibrous Plant remains recognizable and retains some strength Plant remains recognizable and open structure Spongy Very compressible and open structure Strength lost Very organic sand Strength lost Very organic sand Strength lost Very organic sand Spongy Spon		C <sub>u</sub> 150 - 300kP Hard (or very weak mudstone)	Can be scratched by						Very (sandy <sup>f)</sup> )	>65 <sup>a)</sup>				Sticks to the fingers and dries slowly; Shrinks appreciably on drying usually			
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	rgani	Spongy					Very	organic sand	Bla	ck	0,						
Plastic and smears fingers Amorphous absent Peat Contain disseminated or discrete mineral soils.	0		Can be moulded in har and smears fingers	nd Amo					bla	ck in colour, d	listinctive smell, lov	bulk density. Can					





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4 =						<u> </u>		
		STRATA						& TESTS
Depth No 0.00-0.40	TOPSOIL: Brown silty cl	DESCRI	PTION			Depth	n No	Remarks/Tests
0.00-0.40	TOPSOIL. BIOWITSHLY CI	ay with frequent rootiets.						
0.40-2.30	Firm orangey brown mot	tled blue silty CLAY.				0.50	J	
	1.10bluish grey					1.00	VANE D	90
2								
2.30-2.70	(IVIedium dense) bluish g	grey clayey gravelly SAND.						
2.70-3.00	Firm bluish grey silty CL	AY.						
Shoring/Supp	port: None.						NERAL	
Stability: Sta	ble. : Slight groundwater :	seepage at 2.70m.				RE	MARKS	
) [	3.00 — H  A  B 0.70	200pago at 2.7 om.						
All dimensions Scale 1:		Fry & Son	Method/ J Plant Used	ICB 8080 (8T excavat			Logged By	SB



Project Project										AL PIT No
1	k Farm, Gi	lingham, Do	rset							
Job No		Date		Ground Level (r	m) Co	o-Ordinates ()				TP08
09 Contractor	157	25-01	-10						Sheet	
Contractor										1 of 1
	Α		В		С		D			_egend
2	^									
4 —			S	 ΓRΑΤΑ				<u> </u>	AMPI FS	& TESTS
Depth	No			DESCR	IPTION			Dept		Remarks/Tests
0.00-0.40	TOPS	OIL: Brown silty	clay with fre	quent rootlets.						
0.40-1.90	Firm o	rangey brown m	nottled bluish	grey silty CLAY.				0.75	J VANE	70
		gravelly						1.00	VAIVE	70
1.90-2.90	Stiff bl	uish grey silty C	LAY.					2.00	D	
Shoring/S Stability:	Support: N Stable.	one. encountered	1						ENERAL EMARKS	
Groundw	2.90	encountered ——≽	A.							
	A	B 0.70	)							
	С									
All dimens	sions in metre ale 1:50	Client: C.C	G. Fry & So	on	Method/ Plant Used	JCB 8080 (8 excava	BT tracked ator)		Logged By	SB



Project Project										AL PIT No
1	k Farm, C	Gillingham, Do	rset							
Job No		Date		Ground Level (r	n) C	o-Ordinates ()				ГР09
09 Contractor	157	25-01	I-10						Sheet	
Contractor										1 of 1
0 -	F	١	В		С		D	0	L	egend
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								E		
								E		
4 = -								<u>E</u> 4_		
<b>-</b>	I I		S	TRATA	D=1011					& TESTS
Depth 0.00-0.30	No TOP	SOIL: Brown silty	clay with fre	DESCRI equent rootlets.	PHON			Deptl	n No I	Remarks/Tests
0.30-0.80	Firm	bluish grey silty s	slightly sandy	/ CLAY (possible	MADE GROU	JND).				
0.80-1.90	Firm	orangey brown n	nottled bluish	grey silty CLAY.						
	1.30	slightly gravelly	1							
1.90-2.80	Firm	bluish mottled br	own grey slig	ghtly sandy very g	ravelly CLAY	-				
2										
<u> </u>										
Shoring/S Stability:	Support:   Stable.	None. e encountered							ENERAL EMARKS	
Groundw	ater: Non	e encountered	d.							
<b>∀</b>	2.90 —	<b>&gt;</b>								
	A	D 0 70								
	С	B 0.70	J							
			0 5 0 5		T			Т		
All dimens	sions in meti ale 1:50	es Client: C.0	G. Fry & S	on	Method/ Plant Used	JCB 8080 (8 excava	BT tracked ator)		Logged By	SB



Project									
1	arm, Gillingham, Do	rset					AL PIT No		
Job No	Date	Ground Level	m) Co-	-Ordinates ()			TP10		
0915	7 25-01	-10							
Contractor						Sheet			
	Δ						1 of 1		
2 - 3	A	В	С	D			egend		
4-					<u> </u>				
		STRATA					& TESTS		
Depth No 0.00-0.40		DESCF clay with frequent rootlet.	RIPTION		Dept	h No	Remarks/Tests		
0.00-0.40	TOPSOIL. BIOWITSHLY	ciay with frequent rootiet.							
0.40-0.90	Firm orangey brown si	ilty CLAY.			0.50	J			
0.90-2.20	(Medium dense) orang	gey brown clayey slightly san	dy GRAVEL (po	ossible MADE GROUND	1.00	B VANE	too gravelly		
Shoring/Sup	port: None.				GI	ENERAL			
Stability: St	able. r: None encountered	1.				EMARKS			
Groundwate	- 3.70 — → A B 0.70 C								
All dimensions Scale 1		G. Fry & Son	Method/ Plant Used	JCB 8080 (8T track excavator)	ed	Logged By	SB		



Project TR											AL DITAL
	-	Form Cilli	ngham, Do	root						IRI	AL PIT No
	Job No		Date	1361	Ground Level (r	n) Co	o-Ordinates ()			-	TP11
	0915	57	25-01	-10	(		()				
	Contractor									Sheet	
											1 of 1
	0 —	Α		В		С		D	0	L	_egend
										2×	
	=								E		× × × ×
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	· -			E	<u> </u>	<del>* * *</del> *					
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										<u> </u>	<del></del>
										<del> </del>	× × × ×
									E	<u>×</u>  -	× × ×
	3 —								_ 3		
	=								F		
	=								E		
	4-								<u>E</u> 4_		
				S	TRATA				1		& TESTS
	Depth N 0.00-0.20	lo TOPSO	I · Prown cilty	clay with fro	DESCR equent rootlets.	IPTION			Dept	h No	Remarks/Tests
	0.20-1.00		ngey brown si						0.25	J	
	1.00-2.90	Stiff blui	sh grey mottle	d orangey b	rown silty CLAY.				1.00	VANE	100
									2.00	D	
2									2.00		
72											
5		2.70g	ravelly								
ב ב											
5											
<u>.</u>							1				
	Shoring/Su Stability: S	ipport: No	ne.							ENERAL EMARKS	
	Groundwat	er: None e	ncountered	d.					KE		
,   }	<b> </b>	— 3.10 ——									
	'	A.									
	D		B 0.70	1							
200		С									
<u>.</u>						ı					
2001	All dimensior Scale	ns in metres 1:50	Client: C.C	G. Fry & S	on	Method/ Plant Used	JCB 8080 (8 excava	BT tracked ator)		Logged By	SB



Project	rax. 01392 (								TDI	AL PIT No
1 -	Farm. Gilli	ngham, Do	rset							
Job No		Date		Ground Level (r	n) Co	o-Ordinates ()			<b>_</b>	TP12
091	57	25-01	-10							
Contractor					·				Sheet	
										1 of 1
2	A		В	IRATA	C		D	0		egend
Donth	No		S	DESCRI	DTION			Dept		& TESTS Remarks/Tests
Depth 1 0.00-0.30		L: Brown silty	clay with fre	quent rootlets.	PTION			Всрі	140	Terrains/Tests
0.30-0.70	Firm ora	ngey brown si	Ity CLAY.							
0.70-3.00	Ctiff bloo	sh grey silty C	LAY							
0.70-3.00	Sun blui	Sit grey silty C	LAT.					0.75 1.00	J VANE	90
	1.10 - 1.	70mottled b	rown					1.00	VAINE	90
								1.50	D	
Shoring/Su	upport: No	ne.							ENERAL	
Stability: S	Stable.	ncountered	1						EMARKS	
Significan		incountered	1-							
<u></u>	— 3.20 —	<b>──</b>								
	A T									
	D B 0.70									
	С	<u>+</u>								
	ons in metres e 1:50	Client: C.C	3. Fry & So	on	Method/ Plant Used	JCB 8080 (8	BT tracked ator)		Logged By	SB



Fa	x: 01392 678083							
Project	O 1 D						TR	IAL PIT No
Job No	arm, Gillingham, Dor	set Ground L	ovol (m)	Co-Ordinates ()			-	TP13
09157			Lever (III)	Co-Ordinates ()				
Contractor	2001	10					Sheet	
								1 of 1
0 ————————————————————————————————————	A	В			D			Legend
						Ė,		
4		STRATA				4 SA	MPLES	& TESTS
Depth No 0.00-0.40	TOPSOIL: Brown silty		ESCRIPTION			Depth	No	Remarks/Tests
1.50-2.20	Firm orangey brown, m  1.00stiff  (Medium dense) bluish					1.00	J VANE	110
2.20-2.80	Firm to stiff bluish grey	CLAY.				2.00	D	
Shoring/Supp Stability: Sta	able.						NERAL MARKS	
D	2.50 B 0.70	i. Fry & Son	Masha	V IOD 2000 W	OT trools of	1,	organi Di	( OB
All dimensions		i. riy & 3011	Method Plant U	JCB 8080 (8	s i tracked		ogged By	/ SB



Project		1392 07								TRI	AL PIT No
1	k Farm	, Gillin	gham, Doi	rset							
Job No		С	ate		Ground Level (r	n) C	o-Ordinates ()				TP14
	157		25-01	-10							
Contractor										Sheet	4 -£ 4
		Λ									1 of 1
2 - 3 - 3 - 3 - 3 - 3 - 3 - 3 - 3 - 3 -		A		В		С		D			egend
4 =									<u> </u>		
				S	TRATA						& TESTS
Depth 0.00-0.20	No To	DPSOIL :	Brown eilty	clay with fre	DESCRI equent rootlets.	PTION			Deptl	n No	Remarks/Tests
0.20-0.80	Fii	rm brow	n silty slightly	y gravelly CL	AY (possible MA				0.25	J	
		·	•	G ,	, ,	, c			1.00	VANE	too gravelly
1.90-3.00	St	tiff bluish	grey silty Cl	LAY.					1.50	D	
									_		
Shoring/S Stability:	Support Stable	: None	e. countered							ENERAL EMARKS	
Grounaw	3.00 A		B 0.70								
All dimens	sions in male 1:50	netres	Client: C.C	G. Fry & S	on	Method/ Plant Used	JCB 8080 (8 excava	T tracked ator)		Logged By	SB



Project	Project TRIAL PIT No						
	arm, Gillingham, Do	rset					
Job No	Date	Ground Leve	el (m) Co	o-Ordinates ()		<b>-</b>	TP15
09157	7 25-01	I-10					
Contractor						Sheet	
							1 of 1
	A	В	С			\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	egend
STRATA SAMPLES &						& TESTS	
Depth No			CRIPTION		Depth	n No	Remarks/Tests
	D.00-0.30 TOPSOIL: Brown silty clay with frequent rootlets.  D.30-0.90 Firm orangey brown mottled greyish blue silty CLAY with occasional pieces of gravel.						
0.30-0.90	Firm orangey brown m	nottled greyish blue silty CL	AY with occasio	nal pieces of gravel.	0.50	J	
0.90-2.80	Firm to stiff bluish gre	y silty CLAY.			1.00	D	7-
	1.50 - 2.10mottled	brown			1.00	VANE	75
Shoring/Sup	port: None.					ENERAL	
Stability: Stability: Stability: Stability:	able. r: None encountered	d.			RE	MARKS	
D	- 2.90 → A B 0.70	)					
	C						
All dimensions Scale 1		G. Fry & Son	Method/ Plant Used	JCB 8080 (8T track excavator)	ed	Logged By	SB



Project	Project TRIAL PIT No								
1 -	arm. Gilling	ham, Dorset							
Job No		ate	Ground Level (r	n) Co	o-Ordinates ()			┪ ‐	TP16
0915	7	25-01-10							
Contractor								Sheet	
									1 of 1
	A	В		С		D			Legend  The state of the state
4-	$\frac{1}{4}$								
STRATA         SAMPLES           Depth         No           Depth         No						& TESTS Remarks/Tests			
Depth No.		Brown silty clay with f		PTION			Бери	1 140	TCHIAIRS/163t3
0.30-1.60						0.30	J		
1.60-2.50	0.90very	gravelly f bluish grey silty CLA	Υ.				1.00 1.25	VANE D	too gravelly
Shoring/Sup Stability: St	Shoring/Support: None. Stability: Stable. Groundwater: None encountered.							ENERAL EMARKS	
Groundwate	3.00								
All dimensions Scale 1		Client: C.G. Fry &	Son	Method/ Plant Used	JCB 8080 (8 excava	BT tracked ator)		Logged By	SB



Project							
Park Farm, Gillingham, Dorset	TRIAL PIT No						
Job No Date Ground Level (m) Co-Ordinates ()	TP17						
09157 25-01-10							
Contractor	Sheet						
	1 of 1						
0 A B C D	Legend						
[ ] [ ] [ ] [ ] [ ] [ ] [ ] [ ] [ ] [ ]							
4 -							
STRATA           Depth         No         DESCRIPTION         Description	SAMPLES & TESTS oth No Remarks/Tests						
0.00-0.30 TOPSOIL: Brown silty slightly gravelly clay with frequent rootlets.	NATION TROUBLES						
0.30-1.00 MADE GROUND: Orangey brown silty gravelly clay.							
0.50	J						
1.00-1.20 MADE GROUND: Soft brown silty slightly gravelly clay. 1.00							
1.00-1.20 MADE GROUND: Soft brown silty slightly gravelly clay. 1.20-2.80 Firm to stiff bluish grey mottled orangey brown CLAY.	J						
2.50	D						
Shoring/Support: None.	BENERAL						
Stability: Stable. Groundwater: None encountered.	EMARKS						
<b>□</b> 2.70 — □							
A T							
A B 0.70							
A							



Project	Project TRIAL PIT No						
1	arm, Gillingham, Do	rset					
Job No	Date	Ground Leve	I (m) Co-	-Ordinates ()			ГР18
09157	25-01	-10					
Contractor						Sheet	
	Δ						1 of 1
2-	A	В	С	D			egend
$\begin{bmatrix} 4 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 &$							
STRATA SA						AMPLES	& TESTS
Depth No	TODOOU - December 2116		RIPTION		Depth	n No	Remarks/Tests
1.00-2.80	brickwork and ceramics.  0.50					J D VANE	70
	2.30gravelly						
Shoring/Supp Stability: Sta	ıble.					ENERAL EMARKS	
	Groundwater: None encountered.  A  D  C  B 0.70						
All dimensions Scale 1:		S. Fry & Son	Method/ Plant Used	JCB 8080 (8T tracke excavator)	d	Logged By	SB



Fax:	. 01392 070003							
Project							TR	IAL PIT No
Job No	rm, Gillingham, Dor Date	· · · · · · · · · · · · · · · · · · ·	Level (m)	Co-Ordinates ()			-	TP19
09157	25-01-		Levei (III)	Co-Ordinates ()				
Contractor	20 01	10					Sheet	
								1 of 1
2 —	A	В	C		D			Legend  * * * * * * * * * * * * * * * * * * *
						F ,		
4		STRATA				SAI	MPLES	& TESTS
Depth No			DESCRIPTION			Depth	No	Remarks/Tests
0.00-0.20 0.20-1.00	TOPSOIL: Brown silty of Firm to stiff orangey brown to stiff orangey brown to stiff orangey brown to stiff orangey brown to stiff bluish grey silty CL	own silty CLAY.				0.50 1.00 1.00	J D VANE	130
Shoring/Supp Stability: Stat	ort: None.						NERAL MARKS	
Groundwater:	None encountered  3.10  A  B 0.70  C	. Fry & Son	Method	JCB 8080 (8	BT tracked		ogged By	
All dimensions ii Scale 1:5		,	Method Plant U	sed excava	or liacked	-	ogged b)	, 2R

## **SOAKAWAY TEST RESULTS**



## Soakaway Test Results In Accordance with BRE 365 "Soakaway Design"

Job Title: Park Farm, Gillingham, Dorset

Job No.: 09157

Client: C.G. Fry & Son

Date: Mar-10

Test No. TP02

#### **Trial Pit Dimensions**

Length (m):	3.00
Width (m):	0.70
Depth (m):	2.60
Start Water Level (m):	1.06
Total Depth of Test	1.54

#### **Field Results**

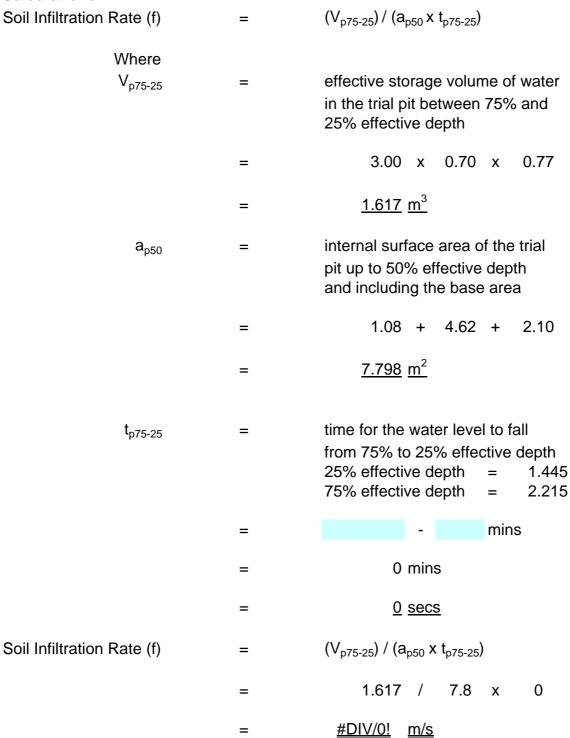
Time (minutes)	Water Level (mBGL)
0	1.06
1	1.06
	1.06
3	1.06
4	1.06
2 3 4 5	1.06
420	1.04

Geotechnical Investigation and Contamination Assessment Report Report Ref: SR/SB/DT/09157/GICAR



## Soakaway Test Results In Accordance with BRE 365 "Soakaway Design"

#### **Calculations**

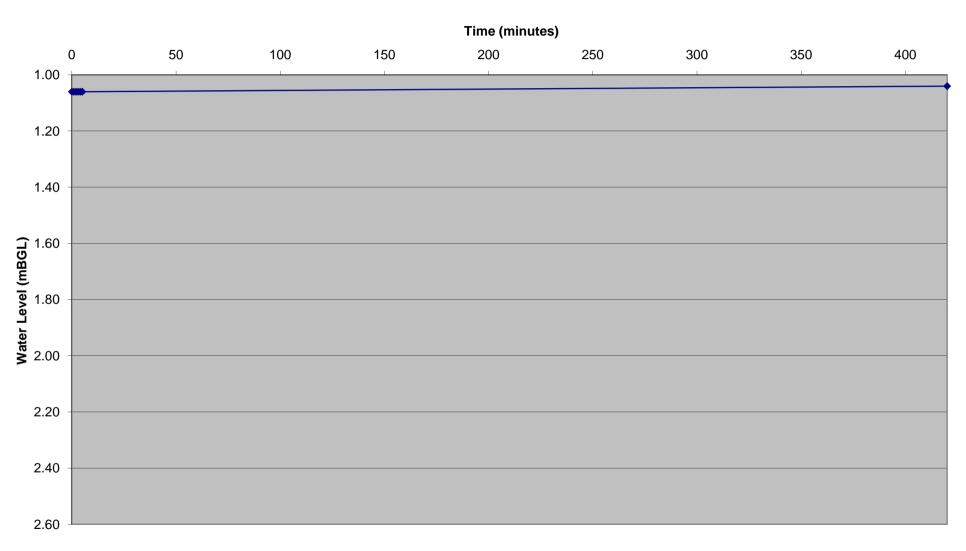


OTHER NOTES:



Park Farm, Gillingham Ruddlesden geotechnical Itd

#### Soakaway Test Results - TP02



Geotechnical Investigation and Contamination Assessment Report Report Ref: SR/SB/DT/09157/GICAR



## Soakaway Test Results In Accordance with BRE 365 "Soakaway Design"

Job Title: Park Farm, Gillingham, Dorset

Job No.: 09157

Client: C.G. Fry & Son

Date: Mar-10

Test No. TP09

#### **Trial Pit Dimensions**

Length (m):	2.90
Width (m):	0.70
Depth (m):	2.80
Start Water Level (m):	1.03
Total Depth of Test	1.77

#### **Field Results**

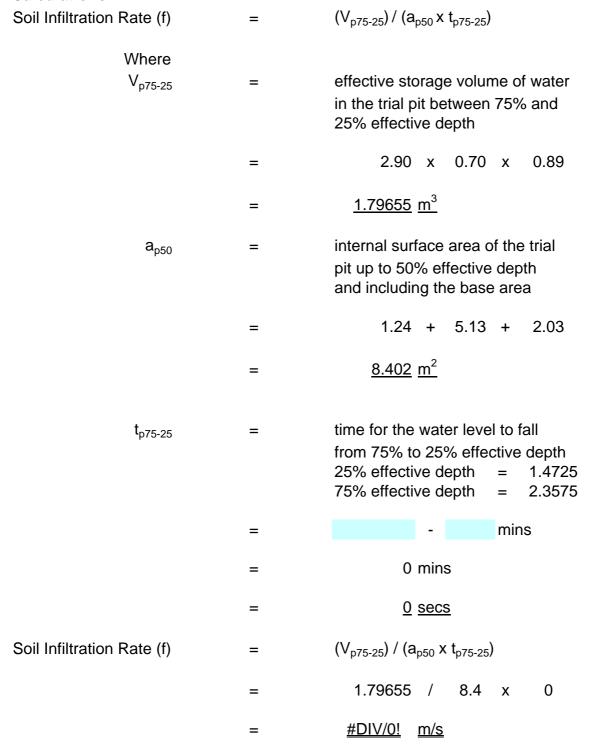
Time (minutes)	Water Level (mBGL)
	vvaler Lever (IIIDGL)
0	1.03
1	1.03
2	1.03
3	1.03
4	1.03
2 3 4 5 372	1.03
372	0.70

Geotechnical Investigation and Contamination Assessment Report Report Ref: SR/SB/DT/09157/GICAR



## Soakaway Test Results In Accordance with BRE 365 "Soakaway Design"

#### **Calculations**

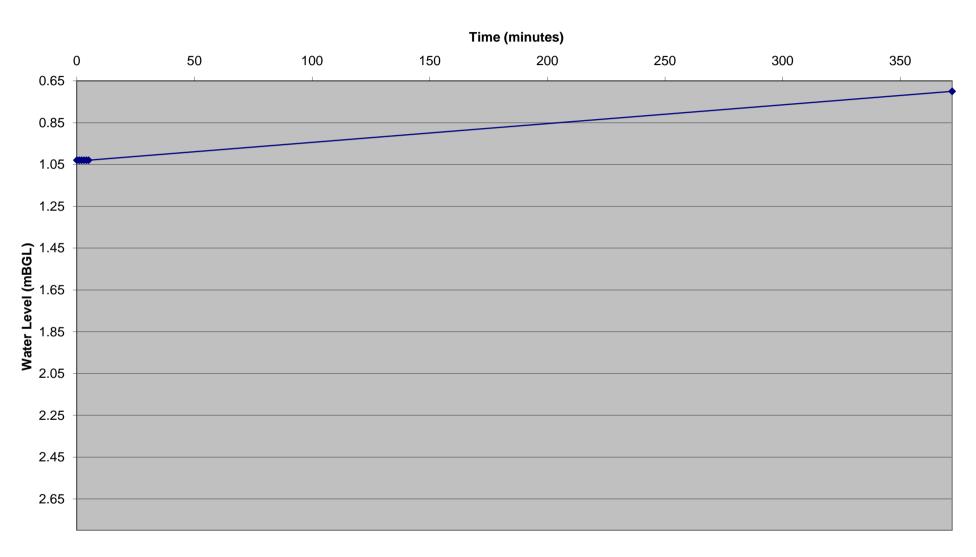


OTHER NOTES: Some collapse of trial pit sides.



Park Farm, Gillingham Ruddlesden geotechnical ltd

#### Soakaway Test Results - TP09







## Soakaway Test Results In Accordance with BRE 365 "Soakaway Design"

Job Title: Park Farm, Gillingham, Dorset

Job No.: 09157

Client: C.G. Fry & Son

Date: Mar-10

Test No. TP15

#### **Trial Pit Dimensions**

Length (m):	2.90
Width (m):	0.70
Depth (m):	2.80
Start Water Level (m):	0.90
Total Depth of Test	1.90

#### **Field Results**

Time (minutes)	Water Level (mBGL)
0	0.90
1	0.90
	0.90
2	
3	0.90
2 3 4 5	0.90
	0.90
308	0.89

Geotechnical Investigation and Contamination Assessment Report Report Ref: SR/SB/DT/09157/GICAR



1.375

## Soakaway Test Results In Accordance with BRE 365 "Soakaway Design"

#### **Calculations**

Soil Infiltration Rate (f)

Where 
$$V_{p75-25}$$
 = effective storage volume of water in the trial pit between 75% and 25% effective depth = 2.90 x 0.70 x 0.95 =  $\frac{1.9285 \text{ m}^3}{1.9285 \text{ m}^3}$  = internal surface area of the trial pit up to 50% effective depth and including the base area =  $\frac{1.33 + 5.51 + 2.03}{1.925}$  =  $\frac{1.33 + 5.51 + 2.03}{1.925}$  =  $\frac{1.33 + 5.51 + 2.03}{1.925}$  =  $\frac{1.33 + 5.51 + 2.03}{1.925}$ 

 $(V_{p75-25})/(a_{p50} \times t_{p75-25})$ 

25% effective depth

#DIV/0! m/s

Soil Infiltration Rate (f) = 
$$(V_{p75-25}) / (a_{p50} \times t_{p75-25})$$
  
= 1.9285 / 8.87 x 0

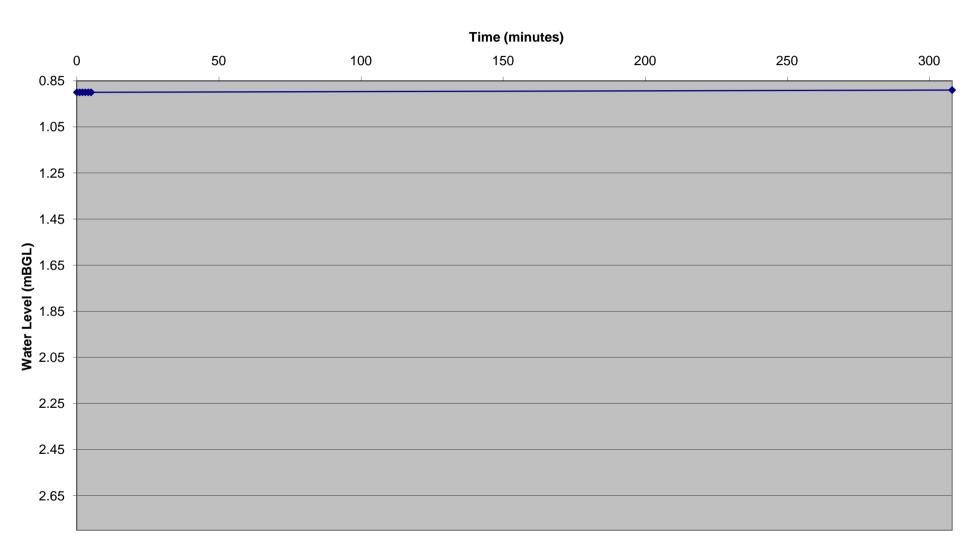
=

OTHER NOTES:



Park Farm, Gillingham Ruddlesden geotechnical Itd

#### **Soakaway Test Results - TP15**







Ruddlesden geotechnical ltd

## **IN-SITU CBR (TRL DCP METHOD) TEST RESULTS**



#### UK DCP V3.1

#### DCP Layer Strength Analysis Report

Project Name: 09157 - Park Farm, Gillingham DCP1

0.000

Surface Type: Thickness (mm):

Unpaved 0

Chainage (km): Direction: Location/Offset:

Carriageway 60 degrees

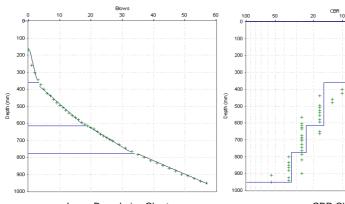
Base Type: Thickness (mm):

Cone Angle: Zero Error (mm): Test Date: 25/01/2010

Surface Moisture: Moisture adjustment factor:

Moderate Not adjusted

Layer Boundaries: Chainage 0.000



#### Layer Boundaries Chart

CBR Chart

#### Layer Properties

No.	Penetration	CBR	Thickness	Depth to	Position	Strength	SN	SNC	SNP
	Rate	(%)	(mm)	layer bottom		Coefficient			
	(mm/blow)			(mm)					
1	73.42	3	361	361	Subgrade				
2	16.41	16	254	615	Subgrade				
3	11.15	24	161	776	Subgrade				
4	7.93	34	174	950	Subgrade				

#### Pavement Strength

	Layer Contribution					
Layer	SN	SNC	SNP			
Surface						
Base						
Sub-Base						
Subgrade		0.13	0.13			
Pavement Strength		0.13	0.13			

CBR Relationship: TRL equation:  $\log_{10}(CBR) = 2.48 - 1.057 \times \log_{10}(Strength)$ 

Report produced by ...

# DCP Layer Strength Analysis Report

Project Name: 09157 - Park Farm, Gillingham DCP2

0.000

Surface Type: Thickness (mm):

Unpaved 0

Chainage (km): Direction: Location/Offset: Cone Angle: Zero Error (mm): Test Date:

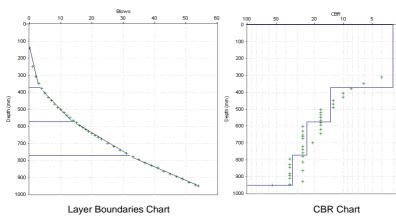
Carriageway 60 degrees 25/01/2010

Base Type: Thickness (mm):

Surface Moisture: Moisture adjustment factor:

Moderate Not adjusted

Layer Boundaries: Chainage 0.000



### Layer Properties

No.	Penetration Rate	CBR (%)	Thickness (mm)	Depth to layer bottom	Position	Strength Coefficient	SN	SNC	SNP
	(mm/blow)			(mm)					
-	77.05	3	371	371	Subgrade				
2	18.89	14	204	575	Subgrade				
3	11.04	24	197	772	Subgrade				
	7.92	34	178	950	Subgrade				

### Pavement Strength

	Layer Contribution						
Layer	SN	SNC	SNP				
Surface							
Base							
Sub-Base							
Subgrade		0.07	0.07				
Pavement Strength		0.07	0.07				

CBR Relationship: TRL equation:  $\log_{10}(CBR) = 2.48 - 1.057 \times \log_{10}(Strength)$ 

# DCP Layer Strength Analysis Report

Project Name: 09157 - Park Farm, Gillingham DCP3

Chainage (km): Direction: Location/Offset:

Surface Type: Thickness (mm):

Unpaved 0

Carriageway 60 degrees

0.000

Base Type: Thickness (mm):

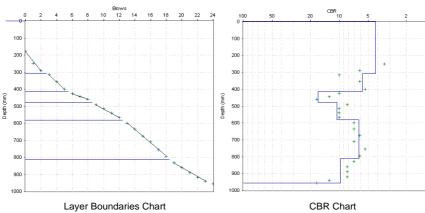
Cone Angle: Zero Error (mm): Test Date: 25/01/2010

Surface Moisture: Moisture adjustment factor:

Moderate

Not adjusted

### Layer Boundaries: Chainage 0.000



### Layer Properties

No.	Penetration	CBR	Thickness	Depth to	Position	Strength	SN	SNC	SNP
	Rate	(%)	(mm)	layer bottom		Coefficient			
	(mm/blow)			(mm)					
1	56.80	4	308	308	Subgrade				
2	42.60	6	106	414	Subgrade				
3	15.46	17	63	477	Subgrade				
4	23.87	11	105	582	Subgrade				
5	38.95	6	230	812	Subgrade				
6	25.91	10	143	955	Subgrade				

### Pavement Strength

	Layer Contribution						
Layer	SN SNC SNP						
Surface							
Base							
Sub-Base							
Subgrade		0.43	0.43				
Pavement Strength		0.43	0.43				

CBR Relationship: TRL equation:  $\log_{10}(CBR) = 2.48 - 1.057 \times \log_{10}(Strength)$ 

# DCP Layer Strength Analysis Report

Project Name: 09157 - Park Farm, Gillingham DCP4

0.000

Surface Type: Thickness (mm):

Unpaved 0

Chainage (km): Direction: Location/Offset:

Carriageway 60 degrees

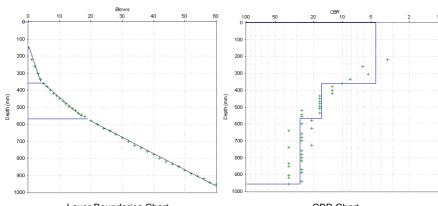
Base Type: Thickness (mm):

Cone Angle: Zero Error (mm): Test Date: 25/01/2010

Surface Moisture: Moisture adjustment factor:

Moderate Not adjusted

Layer Boundaries: Chainage 0.000



Layer Boundaries Chart

CBR Chart

### Layer Properties

No.	Penetration Rate	CBR (%)	Thickness (mm)	Depth to layer bottom	Position	Strength Coefficient	SN	SNC	SNP
	(mm/blow)			(mm)					
1	53.52	4	359	359	Subgrade				
2	15.73	16	209	568	Subgrade				-
3	9.58	28	387	955	Subgrade				

### Pavement Strength

	Layer Contribution						
Layer	SN	SNC	SNP				
Surface							
Base							
Sub-Base							
Subgrade		0.50	0.50				
Pavement Strength		0.50	0.50				

CBR Relationship: TRL equation:  $\log_{10}(CBR) = 2.48 - 1.057 \times \log_{10}(Strength)$ 

# DCP Layer Strength Analysis Report

Project Name: 09157 - Park Farm, Gillingham DCP5

Chainage (km): Direction: Location/Offset:

0.000

Surface Type: Thickness (mm):

Unpaved 0

Carriageway 60 degrees

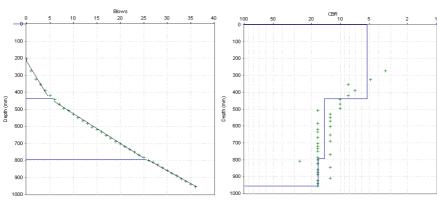
Base Type: Thickness (mm):

Cone Angle: Zero Error (mm): Test Date: 25/01/2010

Surface Moisture: Moisture adjustment factor:

Moderate Not adjusted

Layer Boundaries: Chainage 0.000



Layer Boundaries Chart

CBR Chart

### Layer Properties

No.	Penetration	CBR	Thickness	Depth to	Position	Strength	SN	SNC	SNP
	Rate	(%)	(mm)	layer bottom		Coefficient			
	(mm/blow)			(mm)					
1	45.92	5	440	440	Subgrade				
2	17.53	15	354	794	Subgrade				
3	15 17	17	161	955	Subgrade				

### Pavement Strength

	La	yer Contribut	ion
Layer	SN	SNC	SNP
Surface			
Base			
Sub-Base			
Subgrade		0.66	0.66
Pavement Strength		0.66	0.66

CBR Relationship: TRL equation:  $\log_{10}(CBR) = 2.48 - 1.057 \times \log_{10}(Strength)$ 

# DCP Layer Strength Analysis Report

Project Name: 09157 - Park Farm, Gillingham DCP6

0.000

Surface Type: Thickness (mm):

Unpaved 0

Chainage (km): Direction: Location/Offset: Cone Angle: Zero Error (mm): Test Date:

Carriageway 60 degrees

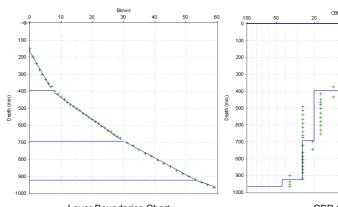
Base Type: Thickness (mm):

25/01/2010

Surface Moisture: Moisture adjustment factor:

Moderate Not adjusted





Layer Boundaries Chart

CBR Chart

### Layer Properties

No.	Penetration	CBR	Thickness	Depth to	Position	Strength	SN	SNC	SNP
	Rate	(%)	(mm)	layer bottom		Coefficient			
	(mm/blow)			(mm)					
1	32.09	8	395	395	Subgrade				-
2	12.93	20	298	693	Subgrade				
3	9.91	27	229	922	Subgrade	-			-
4	6.30	43	43	965	Subgrade				

### Pavement Strength

	Layer Contribution						
Layer	SN	SNC	SNP				
Surface							
Base							
Sub-Base							
Subgrade		1.02	1.02				
Pavement Strength		1.02	1.02				

CBR Relationship: TRL equation:  $\log_{10}(CBR) = 2.48 - 1.057 \times \log_{10}(Strength)$ 

# DCP Layer Strength Analysis Report

Project Name: 09157 - Park Farm, Gillingham DCP7

0.000

Surface Type: Thickness (mm):

Unpaved 0

Chainage (km): Direction: Location/Offset:

Carriageway 60 degrees

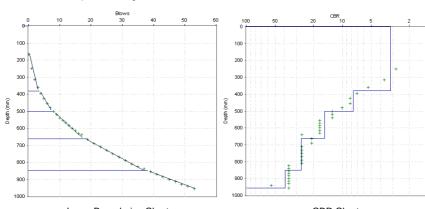
Base Type: Thickness (mm):

Cone Angle: Zero Error (mm): Test Date: 25/01/2010

Surface Moisture: Moisture adjustment factor:

Moderate Not adjusted

Layer Boundaries: Chainage 0.000



Layer Boundaries Chart

CBR Chart

### Layer Properties

No.	Penetration	CBR	Thickness	Depth to	Position	Strength	SN	SNC	SNP
	Rate	(%)	(mm)	layer bottom		Coefficient			
	(mm/blow)			(mm)					
1	75.00	3	381	381	Subgrade				
2	32.22	8	120	501	Subgrade				
3	16.91	15	161	662	Subgrade				
4	9.96	27	188	850	Subgrade				
5	6.91	39	105	955	Subgrade				

### Pavement Strength

	Layer Contribution						
Layer	SN SNC SNP						
Surface							
Base							
Sub-Base							
Subgrade		0.11	0.11				
Pavement Strength		0.11	0.11				

CBR Relationship: TRL equation:  $\log_{10}(CBR) = 2.48 - 1.057 \times \log_{10}(Strength)$ 

# DCP Layer Strength Analysis Report

Project Name: 09157 - Park Farm, Gillingham DCP8

0.000

Surface Type: Thickness (mm):

Unpaved 0

Chainage (km): Direction: Location/Offset:

Carriageway 60 degrees

Base Type: Thickness (mm):

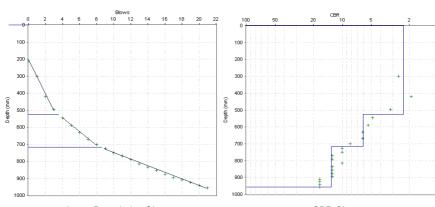
Cone Angle: Zero Error (mm): Test Date:

25/01/2010

Surface Moisture: Moisture adjustment factor:

Moderate Not adjusted

Layer Boundaries: Chainage 0.000



Layer Boundaries Chart

CBR Chart

### Layer Properties

No.	Penetration	CBR	Thickness	Depth to	Position	Strength	SN	SNC	SNP	ı
	Rate	(%)	(mm)	layer bottom		Coefficient				
	(mm/blow)			(mm)						
1	100.34	2	524	524	Subgrade					ı
2	40.33	6	192	716	Subgrade					ı
3	19.50	13	239	955	Subgrade					ı

### Pavement Strength

	La	yer Contribut	ion
Layer	SN	SNC	SNP
Surface			
Base			
Sub-Base			
Subgrade		0.00	0.00
Pavement Strength		0.00	0.00

CBR Relationship: TRL equation:  $\log_{10}(CBR) = 2.48 - 1.057 \times \log_{10}(Strength)$ 

# APPENDIX B PHOTOGRAPHS





**PLATE 1** 

The northwestern area of the site.



PLATE 2

The northeastern area of the site.





## PLATE 3

The central area of the site, viewed from the central—eastern part of the site.



## PLATE 4

The centraleastern area of the site, showing the presence of an attenuation pond.





## PLATE 5

The southeastern area of the site, viewed from the southwest.



## **PLATE 6**

The western area of the site, showing the presence of allotments, hutches and pens.





## PLATE 7

The southwestern area of the site, viewed from the south.



## **PLATE 8**

Ground
conditions
encountered in
TP6, typical of
ground
conditions
encountered
across the site.



# APPENDIX C LABORATORY TESTING RESULTS



# **GEOTECHNICAL LABORATORY TESTING**





Job: Park Farm, Gillingham

Job No: 4423 Client Job No: 09157

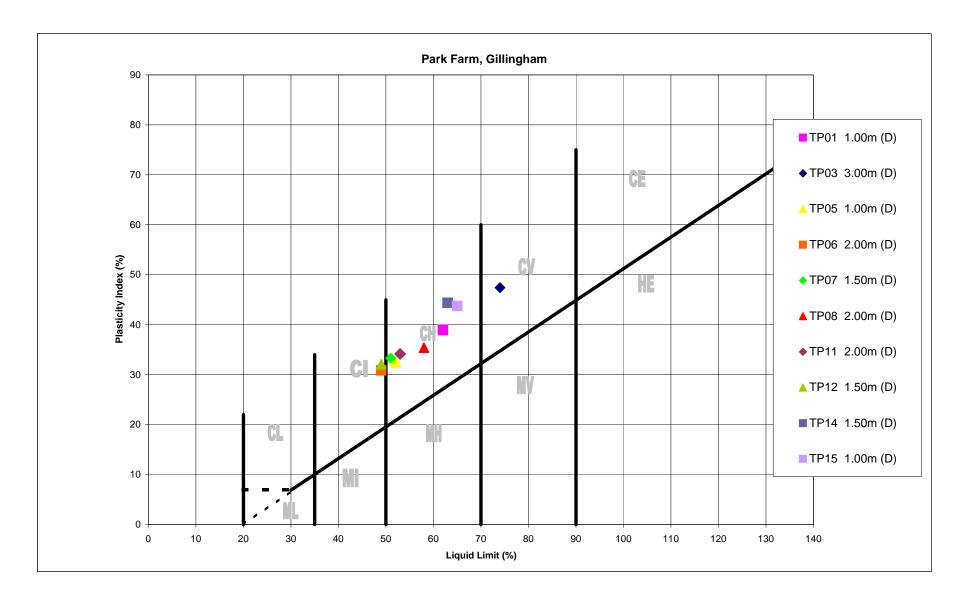
Sample Reference	Natural MC (%)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	% Passing .425mm	Modified Plasticity Index (%)	Preparation Method	Description/ Remarks
TP01 1.00m (D)	27.7	62	23	39	100.0	39	Natural	Grey silty CLAY
TP03 3.00m (D)	35.6	74	27	47	100.0	47	Natural	Grey/black silty CLAY
TP05 1.00m (D)	22.8	52	19	33	100.0	33	Natural	Brown/yellow silty slightly sandy CLAY
TP06 2.00m (D)	27.6	49	18	31	100.0	31	Natural	Brown/grey silty slightly sandy CLAY
TP07 1.50m (D)	26.6	51	18	33	100.0	33	Natural	Brown/green silty slightly sandy CLAY
TP08 2.00m (D)	24.4	58	23	35	100.0	35	Natural	Grey silty CLAY
TP11 2.00m (D)	25.4	53	19	34	100.0	34	Natural	Yellow/brown silty slightly sandy CLAY
TP12 1.50m (D)	29.0	49	17	32	100.0	32	Natural	Grey/yellow silty CLAY
TP14 1.50m (D)	22.7	63	19	44	100.0	44	Natural	Yellow/grey silty/sandy CLAY
TP15 1.00m (D)	28.2	65	21	44	100.0	44	Natural	Grey silty CLAY

Tests carried out in accordance with Clauses 3.2, 4.3, 5.3 and 5.4 of BS1377: Part 2: 1990

Modified Plasticity Index is defined in NHBC Chapter 4.2 as the PI multiplied by the percentage of particles passing the .425mm sieve. Non-Modified Plasticity Indices plotted on the attached Casagrande Classification chart.

 Prepared By: DA
 Date: 29/01/2010
 Processed By: MD
 Date: 05/02/2010







Job: Park Farm, Gillingham

Job No: 4423

Client Job No: 09157

Sample Reference	Natural MC (%)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	% Passing .425mm	Modified Plasticity Index (%)	Preparation Method	Description/ Remarks
TP15 1.00m (D)	28.2	65	21	44	100.0	44	Natural	Grey silty CLAY
TP17 2.50m (D)	27.5	60	21	39	100.0	39	Natural	Grey silty CLAY
TP18 1.00m (D)	19.6	74	25	49	100.0	49	Natural	Green silty CLAY
TP19 1.00m (D)	25.7	57	21	36	100.0	36	Natural	Brown/green silty slightly sandy CLAY

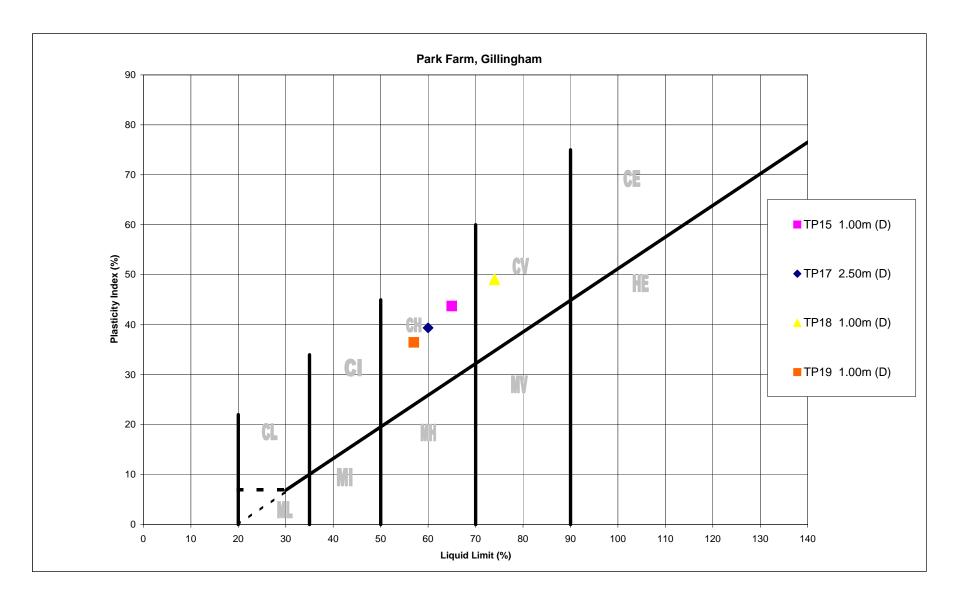
Tests carried out in accordance with Clauses 3.2, 4.3, 5.3 and 5.4 of BS1377: Part 2: 1990

Modified Plasticity Index is defined in NHBC Chapter 4.2 as the PI multiplied by the percentage of particles passing the .425mm sieve.

Non-Modified Plasticity Indices plotted on the attached Casagrande Classification chart.

 Prepared By: DA
 Date: 29/01/2010
 Processed By: MD
 Date: 05/02/2010







Job: Park Farm, Gillingham

Job No: 4423 Client Job No: 09157

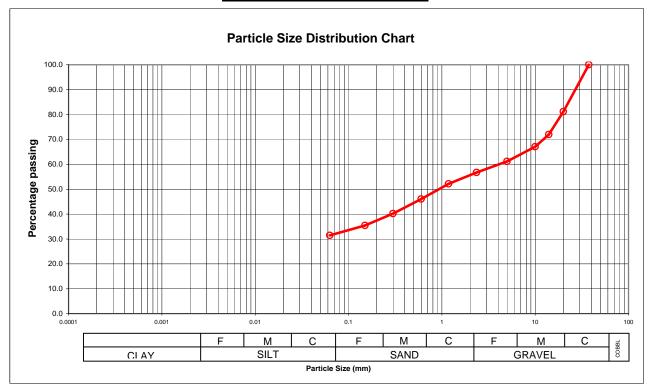
### TP04 2.50m (B)

Green very clayey/silty SAND/GRAVEL

Natural moisture content = 24.4%

Uniformity Coefficient = n/a

Particle size (mm)	% Passing
37.5	100.0
20	81.2
14	71.9
10	67.0
5	61.1
2.36	56.7
1.18	52.2
0.6	46.1
0.3	40.2
0.15	35.4
0.063	31.5



Sample tested in accordance with BS1377: 1990: Part 2: 9.2

Due to size of sample limitations, the BS 1377 recommended sample size for gravel/cobble soils cannot always be achieved.

Prepared By: MD Date: 29/01/2010 Processed By: MD Date: 05/02/2010



Job: Park Farm, Gillingham Job No: 4423 Client Job No: 09157

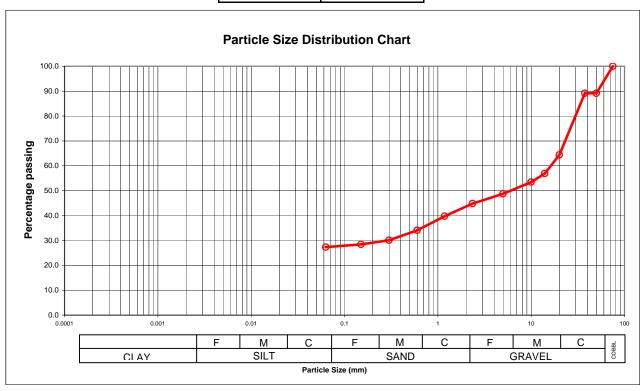
### TP10 1.00m (B)

Green very clayey/silty/sandy fine to coarse GRAVEL

Natural moisture content = 12.6%

Uniformity Coefficient = n/a

Particle size (mm)	% Passing
75	100.0
50	89.1
37.5	89.1
20	64.4
14	56.9
10	53.4
5	48.8
2.36	44.8
1.18	39.7
0.6	34.1
0.3	30.1
0.15	28.4
0.063	27.3



Sample tested in accordance with BS1377: 1990: Part 2: 9.2

Due to size of sample limitations, the BS 1377 recommended sample size for gravel/cobble soils cannot always be achieved.

Prepared By: MD Date: 29/01/2010 Processed By: MD Date: 05/02/2010



Job: Park Farm, Gillingham Job No: 4423 Client Job No: 09157

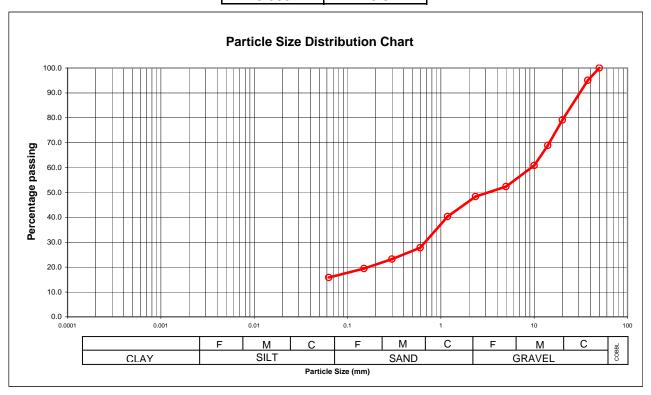
### TP13 2.00m (B)

Green clayey/silty very sandy fine to coarse GRAVEL

Natural moisture content = 21.2%

Uniformity Coefficient = n/a

Particle size (mm)	% Passing
50	100.0
37.5	95.1
20	79.1
14	68.9
10	60.8
5	52.4
2.36	48.4
1.18	40.3
0.6	27.8
0.3	23.2
0.15	19.4
0.063	15.8



Sample tested in accordance with BS1377: 1990: Part 2: 9.2

Due to size of sample limitations, the BS 1377 recommended sample size for gravel/cobble soils cannot always be achieved.

Prepared By: MD Date: 29/01/2010 Processed By: MD Date: 05/02/2010



Job: Park Farm, Gillingham

Job No: 4423 Client Job No: 09157

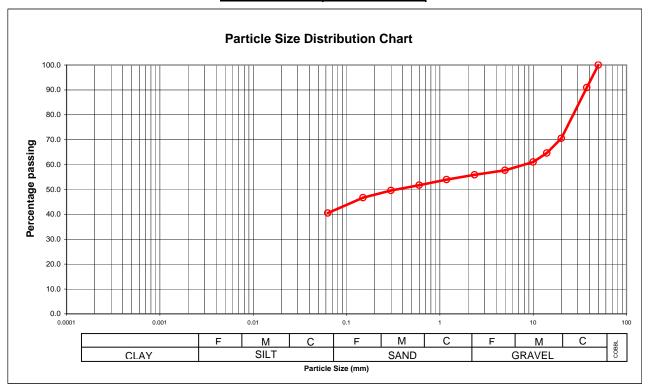
TP16 1.25m (B)

Brown/green very sandy/gravelly CLAY/SILT

Natural moisture content = 21.0%

Uniformity Coefficient = n/a

Particle size (mm)	% Passing
50	100.0
37.5	91.0
20	70.5
14	64.6
10	61.0
5	57.7
2.36	55.9
1.18	53.9
0.6	51.7
0.3	49.6
0.15	46.7
0.063	40.5



Sample tested in accordance with BS1377: 1990: Part 2: 9.2

Due to size of sample limitations, the BS 1377 recommended sample size for gravel/cobble soils cannot always be achieved.

Prepared By: MD Date: 29/01/2010 Processed By: MD Date: 05/02/2010

# **CONTAMINATION LABORATORY TESTING**





Depot Road Newmarket CB8 0AL Tel: 01638 606070

Ruddlesden Geotechnical Ltd 65 Langaton Lane Pinhoe Exeter EX1 3SP

FAO Simon Ruddlesden 09 February 2010

Dear Simon Ruddlesden

Test Report Number 110645

Your Project Reference Park Farm, Gillingham - 09157

Please find enclosed the results of analysis for the samples received 1 February 2010.

All soil samples will be retained for a period of one month and all water samples will be retained for 7 days following the date of the test report. Should you require an extended retention period then please detail your requirements in an email to customerservices@chemtest.co.uk. Please be aware that charges may be applicable for extended sample storage.

If you require any further assistance, please do not hesitate to contact the Customer Services team.

Yours sincerely

□ Darrell Hall Laboratory Manager
□ Phil Hellier Operations Director

□ Keith Jones Technical Development Manager

□ John Crawford Quality Manager
 □ Malcolm Avis Technical Director

Authoriséd Signatory

Notes to accompany report:

The sign < means 'less than'</li>

Tests marked 'U' hold UKAS accreditation

- Tests marked 'M' hold MCertS (and UKAS) accreditation
- Tests marked 'N' do not currently hold UKAS accreditation
- Tests marked 'S' were subcontracted to an approved laboratory
- n/e means 'not evaluated'
- i/s means 'insufficient sample'
- u/s means 'unsuitable sample'
- Comments or interpretations are beyond the scope of UKAS accreditation
- · The results relate only to the items tested

Test Report 110645 Cover Sheet

FAO Simon Ruddlesden

# LABORATORY TEST REPORT



Results of analysis of 35 samples received 01 February 2010

Park Farm, Gillingham - 09157

09 February 2010

Login I	Batch No							110	645			
	est LIMS ID				AE65041	AE65042	AE65043	AE65044	AE65045	AE65046	AE65047	AE65048
Sample					TP01	TP01	TP03	TP03	TP04	TP04	TP05	TP05
Sample												
Depth					1m	1m	0.5m	3m	0.25m	2.5m	0.5m	1m
Matrix					SOIL							
SOP↓	Determinand↓	CAS No↓	Units↓	*								
2625	Organic matter		%	M	1.0		1.6		1.7		1.0	
2120	Boron (hot water soluble)	7440428	mg kg-1	M	1.4		0.8		0.8		0.5	
	Sulfate (2:1 water soluble) as SO4	14808798	g l-¹	M	0.07	0.08	<0.01	0.01	0.01	0.02	<0.01	<0.01
2430	Sulfate (total)	14808798	%	М	<0.01		0.02		<0.01		<0.01	
2450	Arsenic	7440382	mg kg-1	М	<2.0		4.3		13		9.6	
	Cadmium	7440439	mg kg-1	М	<0.10		0.13		<0.10		0.15	
	Chromium	7440473	mg kg-1	М	27		32		49		31	
	Copper	7440508	mg kg-1	М	85		20		14		20	
	Mercury	7439976	mg kg-1	М	<0.10		<0.10		<0.10		<0.10	
	Nickel	7440020	mg kg-1	М	28		21		21		30	
	Lead	7439921	mg kg-1	М	54		28		34		22	
	Selenium	7782492	mg kg-1	М	<0.20		<0.20		<0.20		<0.20	
	Zinc	7440666	mg kg-1	М	88		68		68		58	
2670	Total Petroleum Hydrocarbons		mg kg-1	М	< 10		< 10		< 10		< 10	
2700	Naphthalene	91203	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Acenaphthylene	208968	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Acenaphthene	83329	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Fluorene	86737	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Phenanthrene	85018	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Anthracene	120127	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Fluoranthene	206440	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Pyrene	129000	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Benzo[a]anthracene	56553	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Chrysene	218019	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Benzo[b]fluoranthene	205992	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Benzo[k]fluoranthene	207089	mg kg-1	M	< 0.1		< 0.1		< 0.1		< 0.1	
	Benzo[a]pyrene	50328	mg kg-1	M	< 0.1		< 0.1		< 0.1		< 0.1	
	Dibenzo[a,h]anthracene	53703	mg kg-1	M	< 0.1		< 0.1		< 0.1		< 0.1	
	Indeno[1,2,3-cd]pyrene	193395	mg kg-1	M	< 0.1		< 0.1		< 0.1		< 0.1	
	Benzo[g,h,i]perylene	191242	mg kg-1	M	< 0.1		< 0.1		< 0.1		< 0.1	
	Total (of 16) PAHs	101212	mg kg-1	M	< 2		< 2		< 2		< 2	

All tests undertaken between 02-Feb-2010 and 8-Feb-2010

Column page 1 Report page 1 of 2

<sup>\*</sup> Accreditation status

FAO Simon Ruddlesden

# LABORATORY TEST REPORT



Results of analysis of 35 samples received 01 February 2010

Park Farm, Gillingham - 09157

Report Date 09 February 2010

Login	Batch No							111	0645			
	test LIMS ID				AE65049	AE65050	AE65051	AE65052	AE65053	AE65054	AE65055	AE65056
Sampl					TP06	TP06	TP07	TP07	TP08	TP08	TP10	TP10
Sampl					11.00	11.00	11.01	11.07	11.00	11 00	11.10	
Depth					0.25m	2m	0.5m	1.5m	0.75m	2m	0.5m	1m
Matrix					SOIL							
SOP↓	Determinand↓	CAS No↓	Units↓									
2625	Organic matter		%	M	0.79		0.53		0.59		0.57	
2120	Boron (hot water soluble)	7440428	mg kg-1	М	0.5		0.4		0.6		<0.4	
	Sulfate (2:1 water soluble) as SO4	14808798	g l-¹	М	<0.01	<0.01	<0.01	<0.01	<0.01	0.01	<0.01	0.07
2430	Sulfate (total)	14808798	%	М	<0.01		<0.01		<0.01		<0.01	
2450	Arsenic	7440382	mg kg-1	М	16		14		14		19	
	Cadmium	7440439	mg kg-1	М	0.36		<0.10		<0.10		0.34	
	Chromium	7440473	mg kg-1	М	46		35		33		31	
	Copper	7440508	mg kg-1	М	30		15		31		27	
	Mercury	7439976	mg kg-1	М	<0.10		<0.10		<0.10		<0.10	
	Nickel	7440020	mg kg-1	М	48		10		48		64	
	Lead	7439921	mg kg-1	М	32		22		30		31	
	Selenium	7782492	mg kg-1	М	<0.20		<0.20		0.32		<0.20	
	Zinc	7440666	mg kg-1	М	83		32		98		92	
2670	Total Petroleum Hydrocarbons		mg kg-1	М	< 10		< 10		< 10		< 10	
2700	Naphthalene	91203	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Acenaphthylene	208968	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Acenaphthene	83329	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Fluorene	86737	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Phenanthrene	85018	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Anthracene	120127	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Fluoranthene	206440	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Pyrene	129000	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Benzo[a]anthracene	56553	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Chrysene	218019	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Benzo[b]fluoranthene	205992	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Benzo[k]fluoranthene	207089	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Benzo[a]pyrene	50328	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Dibenzo[a,h]anthracene	53703	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Indeno[1,2,3-cd]pyrene	193395	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Benzo[g,h,i]perylene	191242	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Total (of 16) PAHs		mg kg-1	М	< 2		< 2		< 2		< 2	

All tests undertaken between 02-Feb-2010 and 8-Feb-2010

Column page 2
Report page 1 of 2

<sup>\*</sup> Accreditation status

# LABORATORY TEST REPORT

**Report Date** 

09 February 2010

Results of analysis of 35 samples received 01 February 2010

Park Farm, Gillingham - 09157

FAO Simon Ruddlesden

Login	Batch No							110	645			
Chemt	rest LIMS ID				AE65057	AE65058	AE65059	AE65060	AE65061	AE65062	AE65063	AE65064
Sample	e ID				TP11	TP11	TP12	TP12	TP13	TP13	TP14	TP14
Sample	e No											
Depth					0.25m	2m	0.75m	1.5m	1m	2m	0.25m	1.5m
Matrix					SOIL							
	Determinand↓	CAS No↓	Units↓	*								
	Organic matter		%	M	0.48		1.1		4.7		1.0	
2120	Boron (hot water soluble)	7440428	mg kg-1	M	<0.4		1.0		1.3		<0.4	
	Sulfate (2:1 water soluble) as SO4	14808798	g l-¹	M	<0.01	<0.01	0.10	0.24	0.06	0.03	<0.01	0.01
	Sulfate (total)	14808798	%	M	<0.01		<0.01		<0.01		0.02	
2450	Arsenic	7440382	mg kg-1	M	23		13		72		7.0	
	Cadmium	7440439	mg kg-1	M	0.44		0.50		1.3		<0.10	
	Chromium	7440473	mg kg-1	М	30		44		150		34	
	Copper	7440508	mg kg-1	M	26		38		380		13	
	Mercury	7439976	mg kg-1	M	<0.10		<0.10		0.38		<0.10	
	Nickel	7440020	mg kg-1	М	62		68		180		15	
	Lead	7439921	mg kg-1	М	27		32		240		28	
	Selenium	7782492	mg kg-1	М	0.31		<0.20		<0.20		<0.20	
	Zinc	7440666	mg kg-1	М	90		94		400		51	
2670	Total Petroleum Hydrocarbons		mg kg-1	M	< 10		< 10		< 10		< 10	
2700	Naphthalene	91203	mg kg-1	М	< 0.1		< 0.1		0.41		< 0.1	
	Acenaphthylene	208968	mg kg-1	М	< 0.1		< 0.1		12		< 0.1	
	Acenaphthene	83329	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Fluorene	86737	mg kg-1	М	< 0.1		< 0.1		5.3		< 0.1	
	Phenanthrene	85018	mg kg-1	М	< 0.1		< 0.1		1.1		< 0.1	
	Anthracene	120127	mg kg-1	М	< 0.1		< 0.1		0.42		< 0.1	
	Fluoranthene	206440	mg kg-1	М	< 0.1		< 0.1		0.65		< 0.1	
	Pyrene	129000	mg kg-1	М	< 0.1		< 0.1		0.43		< 0.1	
	Benzo[a]anthracene	56553	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Chrysene	218019	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Benzo[b]fluoranthene	205992	mg kg-1	M	< 0.1		< 0.1		< 0.1		< 0.1	
	Benzo[k]fluoranthene	207089	mg kg-1	M	< 0.1		< 0.1		< 0.1		< 0.1	
	Benzo[a]pyrene	50328	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Dibenzo[a,h]anthracene	53703	mg kg-1	M	< 0.1		< 0.1		< 0.1		< 0.1	
	Indeno[1,2,3-cd]pyrene	193395	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Benzo[g,h,i]perylene	191242	mg kg-1	М	< 0.1		< 0.1		< 0.1		< 0.1	
	Total (of 16) PAHs		mg kg-1	M	< 2		< 2		20		< 2	

All tests undertaken between 02-Feb-2010 and 8-Feb-2010

\* Accreditation status

Column page 3 Report page 1 of 2

# LABORATORY TEST REPORT

Chemtest
The right chemistry to deliver results
Report Date

Results of analysis of 35 samples received 01 February 2010

Park Farm, Gillingham - 09157

Report Date 09 February 2010

FAO Simon Ruddlesden

Login	Batch No							110	1645			
Chem	test LIMS ID				AE65065	AE65066	AE65067	AE65068	AE65069	AE65070	AE65071	AE65072
Sampl	e ID				TP15	TP15	TP16	TP16	TP17	TP17	TP17	TP18
Sampl	e No											
Depth					0.5m	1m	0.3m	1.25m	0.5m	1m	2.5m	0.5m
Matrix					SOIL							
	Determinand↓	CAS No↓	Units↓	*								
	Organic matter		%	M	0.95		0.98		3.3	2.8		2.1
2120	Boron (hot water soluble)	7440428	mg kg-1	M	0.6		<0.4		0.5	<0.4		<0.4
	Sulfate (2:1 water soluble) as SO4	14808798	g l-¹	M	0.02	0.10	<0.01	0.03	0.14	0.04	0.22	0.24
	Sulfate (total)	14808798	%	M	<0.01		<0.01		0.04	0.05		0.19
2450	Arsenic	7440382	mg kg-1	M	8.7		10		13	4.6		7.2
	Cadmium	7440439	mg kg-1	M	0.63		<0.10		<0.10	<0.10		<0.10
	Chromium	7440473	mg kg-1	M	39		32		23	27		30
	Copper	7440508	mg kg-1	M	31		15		13	11		22
	Mercury	7439976	mg kg-1	M	<0.10		<0.10		<0.10	<0.10		<0.10
	Nickel	7440020	mg kg-1	M	44		15		24	17		36
	Lead	7439921	mg kg-1	M	30		21		26	25		38
	Selenium	7782492	mg kg-1	M	<0.20		<0.20		<0.20	<0.20		<0.20
	Zinc	7440666	mg kg-1	M	120		46		81	92		82
2670	Total Petroleum Hydrocarbons		mg kg-1	M	< 10		< 10		< 10	< 10		< 10
2700	Naphthalene	91203	mg kg-1	M	< 0.1		< 0.1		< 0.1	< 0.1		< 0.1
	Acenaphthylene	208968	mg kg-1	М	< 0.1		< 0.1		< 0.1	< 0.1		< 0.1
	Acenaphthene	83329	mg kg-1	M	< 0.1		< 0.1		< 0.1	< 0.1		< 0.1
	Fluorene	86737	mg kg-1	М	< 0.1		< 0.1		< 0.1	< 0.1		< 0.1
	Phenanthrene	85018	mg kg-1	М	< 0.1		< 0.1		< 0.1	< 0.1		0.15
	Anthracene	120127	mg kg-1	М	< 0.1		< 0.1		< 0.1	< 0.1		< 0.1
	Fluoranthene	206440	mg kg-1	М	< 0.1		< 0.1		< 0.1	< 0.1		0.31
	Pyrene	129000	mg kg-1	М	< 0.1		< 0.1		< 0.1	< 0.1		0.25
	Benzo[a]anthracene	56553	mg kg-1	М	< 0.1		< 0.1		< 0.1	< 0.1		0.3
	Chrysene	218019	mg kg-1	М	< 0.1		< 0.1		< 0.1	< 0.1		0.23
	Benzo[b]fluoranthene	205992	mg kg-1	М	< 0.1		< 0.1		< 0.1	< 0.1		0.14
	Benzo[k]fluoranthene	207089	mg kg-1	М	< 0.1		< 0.1		< 0.1	< 0.1		< 0.1
	Benzo[a]pyrene	50328	mg kg-1	M	< 0.1		< 0.1		< 0.1	< 0.1		0.17
	Dibenzo[a,h]anthracene	53703	mg kg-1	M	< 0.1		< 0.1		< 0.1	< 0.1		< 0.1
	Indeno[1,2,3-cd]pyrene	193395	mg kg-1	M	< 0.1		< 0.1		< 0.1	< 0.1		< 0.1
	Benzo[g,h,i]perylene	191242	mg kg-1	M	< 0.1		< 0.1		< 0.1	< 0.1		< 0.1
	Total (of 16) PAHs		mg kg-1	M	< 2		< 2		< 2	< 2		< 2

All tests undertaken between 02-Feb-2010 and 8-Feb-2010

\* Accreditation status

Column page 4
Report page 1 of 2

# LABORATORY TEST REPORT



Results of analysis of 35 samples received 01 February 2010

Park Farm, Gillingham - 09157

FAO Simon Ruddlesden

Login Ba						110645	
Chemtes	st LIMS ID				AE65073	AE65074	AE65075
Sample I					TP18	TP19	TP19
Sample N	No						
Depth					1m	0.5m	1m
Matrix					SOIL	SOIL	SOIL
	Determinand↓	CAS No↓	Units↓	*			
	Organic matter		%	M		2.4	
	oron (hot water soluble)	7440428	mg kg-1	M		1.0	
	ulfate (2:1 water soluble) as SO4	14808798	g l-¹	M	0.13	0.03	0.05
	ulfate (total)	14808798	%	M		0.02	
2450 A	rsenic	7440382	mg kg-1	M		4.2	
С	admium	7440439	mg kg-1	M		<0.10	
С	hromium	7440473	mg kg-1	M		35	
С	copper	7440508	mg kg-1	M		15	
M	lercury	7439976	mg kg-1	М		<0.10	
N	lickel	7440020	mg kg-1	М		23	
Le	ead	7439921	mg kg-1	М		24	
S	elenium	7782492	mg kg-1	М		<0.20	
Zi	inc	7440666	mg kg-1	М		57	
2670 To	otal Petroleum Hydrocarbons		mg kg-1	М		< 10	
2700 N	laphthalene	91203	mg kg-1	М		< 0.1	
A	cenaphthylene	208968	mg kg-1	М		< 0.1	
A	cenaphthene	83329	mg kg-1	М		< 0.1	
F	luorene	86737	mg kg-1	М		< 0.1	
Р	henanthrene	85018	mg kg-1	М		< 0.1	
A	nthracene	120127	mg kg-1	М		< 0.1	
F	luoranthene	206440	mg kg-1	М		< 0.1	
P	yrene	129000	mg kg-1	М		< 0.1	
	enzo[a]anthracene	56553	mg kg-1	М		< 0.1	
_	Chrysene	218019	mg kg-1	М		< 0.1	
	enzo[b]fluoranthene	205992	mg kg-1	M		< 0.1	
	enzo[k]fluoranthene	207089	mg kg-1	M		< 0.1	
	enzo[a]pyrene	50328	mg kg-1	M		< 0.1	
	ibenzo[a,h]anthracene	53703	mg kg-1	M		< 0.1	
	ndeno[1,2,3-cd]pyrene	193395	mg kg-1	M		< 0.1	
	enzo[g,h,i]perylene	191242	mg kg-1	M		< 0.1	
	otal (of 16) PAHs	101272	mg kg-1	M		< 2	

All tests undertaken between 02-Feb-2010 and 8-Feb-2010

<sup>\*</sup> Accreditation status

FAO Simon Ruddlesden

# LABORATORY TEST REPORT



Report Date 09 February 2010

Results of analysis of 35 samples received 01 February 2010

			110645								
			AE65041	AE65042	AE65043	AE65044	AE65045	AE65046	AE65047	AE65048	
			TP01	TP01	TP03	TP03	TP04	TP04	TP05	TP05	
			1m	1m	0.5m	3m	0.25m	2.5m	0.5m	1m	
			SOIL								
2920 Phenols (total)	mg l	rg-1 N	<0.3		<0.3		<0.3		<0.3		
2010 pH	-	M	7.7	7.6	6.0	6.4	6.5	7.8	7.6	7.6	

FAO Simon Ruddlesden

# LABORATORY TEST REPORT



Report Date 09 February 2010

Results of analysis of 35 samples received 01 February 2010

			110645								
			AE65049	AE65050	AE65051	AE65052	AE65053	AE65054	AE65055	AE65056	
			TP06	TP06	TP07	TP07	TP08	TP08	TP10	TP10	
			0.25m	2m	0.5m	1.5m	0.75m	2m	0.5m	1m	
			SOIL								
2920 Phenols (total)	mg kg-1	N	<0.3		<0.3		<0.3		<0.3		
2010 pH	-	M	7.7	7.9	7.8	8.1	7.8	7.8	7.7	8.0	

FAO Simon Ruddlesden

# LABORATORY TEST REPORT

**Report Date** 09 February 2010

Results of analysis of 35 samples received 01 February 2010

			110645								
			AE65057	AE65058	AE65059	AE65060	AE65061	AE65062	AE65063	AE65064	
			TP11	TP11	TP12	TP12	TP13	TP13	TP14	TP14	
			0.25m	2m	0.75m	1.5m	1m	2m	0.25m	1.5m	
			SOIL								
2920 Phenols (total)	mg kg-1	N	<0.3		<0.3		<0.3		<0.3		
2010 pH	-	М	7.9	8.0	8.0	8.0	7.6	7.9	6.7	5.6	

FAO Simon Ruddlesden

# LABORATORY TEST REPORT

**Report Date** 09 February 2010

Results of analysis of 35 samples received 01 February 2010

			110645								
			AE65065	AE65066	AE65067	AE65068	AE65069	AE65070	AE65071	AE65072	
			TP15	TP15	TP16	TP16	TP17	TP17	TP17	TP18	
			0.5m	1m	0.3m	1.25m	0.5m	1m	2.5m	0.5m	
			SOIL								
2920 Phenols (total)	mg kg-1	N	<0.3		<0.3		<0.3	<0.3		<0.3	
2010 pH	-	М	7.7	8.0	7.6	7.8	7.7	7.5	8.0	7.7	

# LABORATORY TEST REPORT

**Report Date** 09 February 2010

Results of analysis of 35 samples received 01 February 2010

Park Farm, Gillingham - 09157

FAO Simon Ruddlesden

					110645	
				AE65073	AE65074	AE65075
				TP18	TP19	TP19
				1m	0.5m	1m
				SOIL	SOIL	SOIL
2920	Phenols (total)	mg kg-1	N		<0.3	
2010	рН	-	М	7.9	7.5	8.2

# Generic Assessment Criteria (GAC) Residential Land Use

Determinand	Unit	GAC	Highest Recorded Value	Location of Highest Recorded Value	No. of values exceeding GAC	Source of GAC
Boron (water soluble)	mg/kg	291	1.4	TP01	0 of 18	LQM/ CIEH
Sulphate (2:1 extract)	g/l	1.2	0.24	TP12	0 of 35	BRE
Sulphate (total)	%	2.0	0.19	TP18	0 of 18	ICRCL
Arsenic	mg/kg	32	72	TP13	1 of 18	SGV
Cadmium	mg/kg	10	1.3	TP13	0 of 18	SGV
Chromium	mg/kg	3000	150	TP13	0 of 18	LQM/ CIEH
Copper	mg/kg	2330	380	TP13	0 of 18	LQM/ CIEH
Mercury	mg/kg	1	0.38	TP13	0 of 18	SGV
Nickel	mg/kg	130	180	TP13	1 of 18	SGV
Lead	mg/kg	450	240	TP13	0 of 18	SGV (OLD)
Selenium	mg/kg	350	0.32	TP08	0 of 18	SGV
Zinc	mg/kg	3750	400	TP13	0 of 18	LQM/ CIEH
Total TPH	mg/kg	50	10	All	0 of 18	DUTCH
Naphthalene	mg/kg	1% 2.5% 6% SOM SOM SOM 1.5 3.7 8.7	0.41	TP13	0 of 18	LQM/ CIEH
Acenaphthylene	mg/kg	1% 2.5% 6% SOM SOM SOM 170 400 850	12	TP13	0 of 18	LQM/ CIEH
Acenaphthene	mg/kg	1% 2.5% 6% SOM SOM SOM 210 480 1000	0.1	All	0 of 18	LQM/ CIEH
Fluorene	mg/kg	1% 2.5% 6% SOM SOM SOM 160 380 780	5.3	TP13	0 of 18	LQM/ CIEH
Phenanthrene	mg/kg	1% 2.5% 6% SOM SOM SOM 92 200 380	1.1	TP13	0 of 18	LQM/ CIEH
Anthracene	mg/kg	1% 2.5% 6% SOM SOM SOM 2300 4900 9200	0.42	TP13	0 of 18	LQM/ CIEH
Fluoranthene	mg/kg	1% 2.5% 6% SOM SOM SOM 260 460 670	0.65	TP13	0 of 18	LQM/ CIEH
Pyrene	mg/kg	1% 2.5% 6% SOM SOM SOM 560 1000 1600	0.43	TP13	0 of 18	LQM/ CIEH
Benzo(a)anthracene	mg/kg	1% 2.5% 6% SOM SOM SOM 3.1 4.7 5.9	0.3	TP18	0 of 18	LQM/ CIEH
Chrysene	mg/kg	1% 2.5% 6% SOM SOM SOM 6.0 8.0 9.3	0.23	TP18	0 of 18	LQM/ CIEH
Benzo(b)fluoranthene	mg/kg	1% 2.5% 6% SOM SOM SOM 5.6 6.5 7.0	0.14	TP18	0 of 18	LQM/ CIEH
Benzo(k)fluoranthene	mg/kg	1% 2.5% 6% SOM SOM SOM 8.5 9.6 10	0.1	All	0 of 18	LQM/ CIEH
Benzo(a)pyrene	mg/kg	1% 2.5% 6% SOM SOM SOM 0.83 0.94 1.0	0.17	TP18	0 of 18	LQM/ CIEH
Dibenzo(a,h) anthracene	mg/kg	1% 2.5% 6% SOM SOM SOM 0.76 0.86 0.90	0.1	All	0 of 18	LQM/ CIEH



Indeno(1,2,3-cd)pyrene	mg/kg	1% 2.5% 6% SOM SOM SOM 3.2 3.9 4.2	0.1	All	0 of 18	LQM/ CIEH
Benzo(g,h,i)perylene	mg/kg	1% 2.5% 6% SOM SOM SOM 44 46 47	0.1	All	0 of 18	LQM/ CIEH
Phenols (total)	mg/kg	420	0.3	All	0 of 18	SGV
pH (less than)	-	5.5	5.6	TP14	0 of 18	BRE

### Generic Assessment Criteria (GAC) Notes:

- 1. Italic entries indicate GAC exceeded.
- 2. Based on a sandy loam soil and 6% SOM (unless otherwise stated), in accordance with Environment Agency guidance.
- 3. Values are rounded to one or two significant figures.

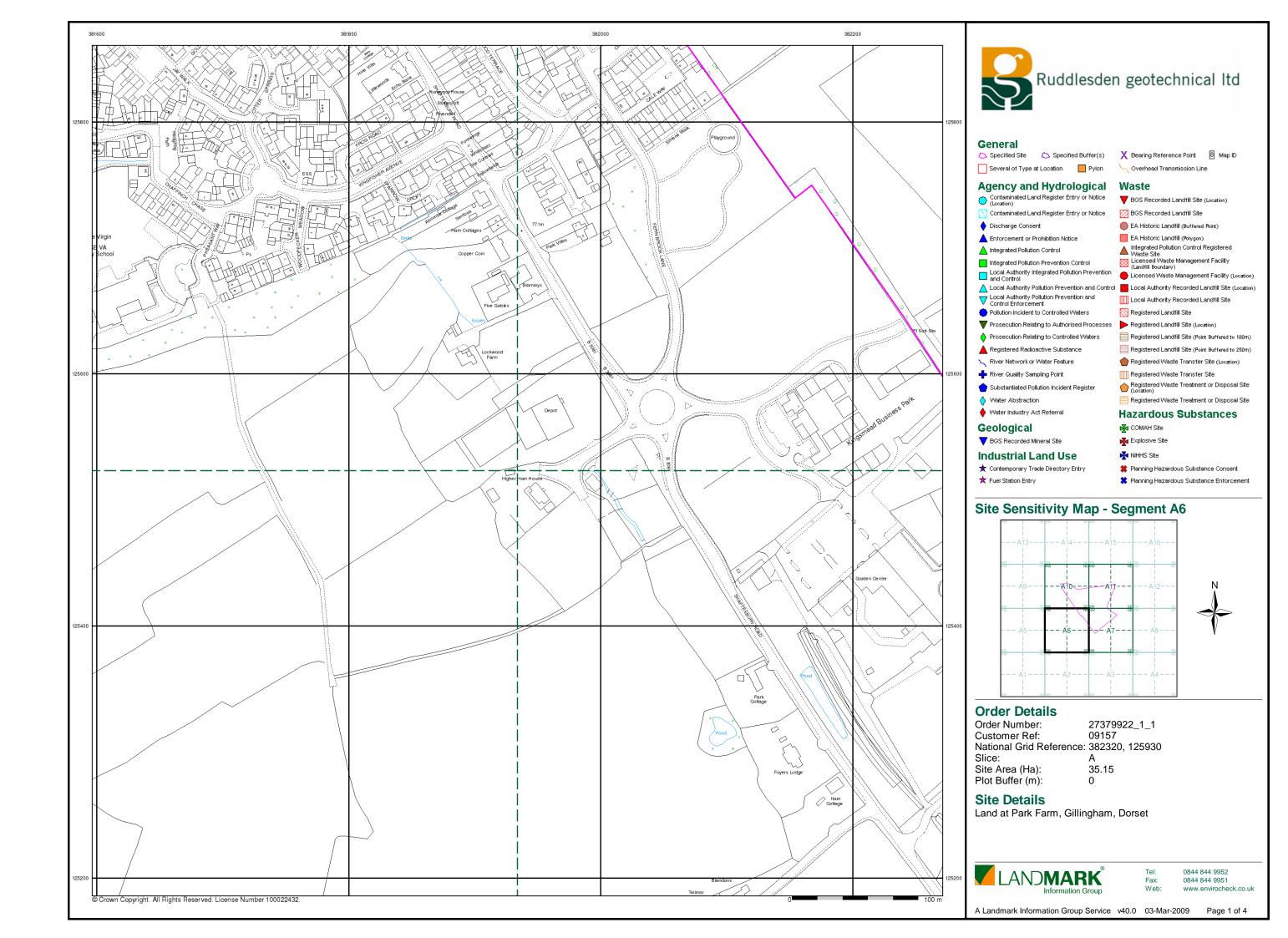
## Key:

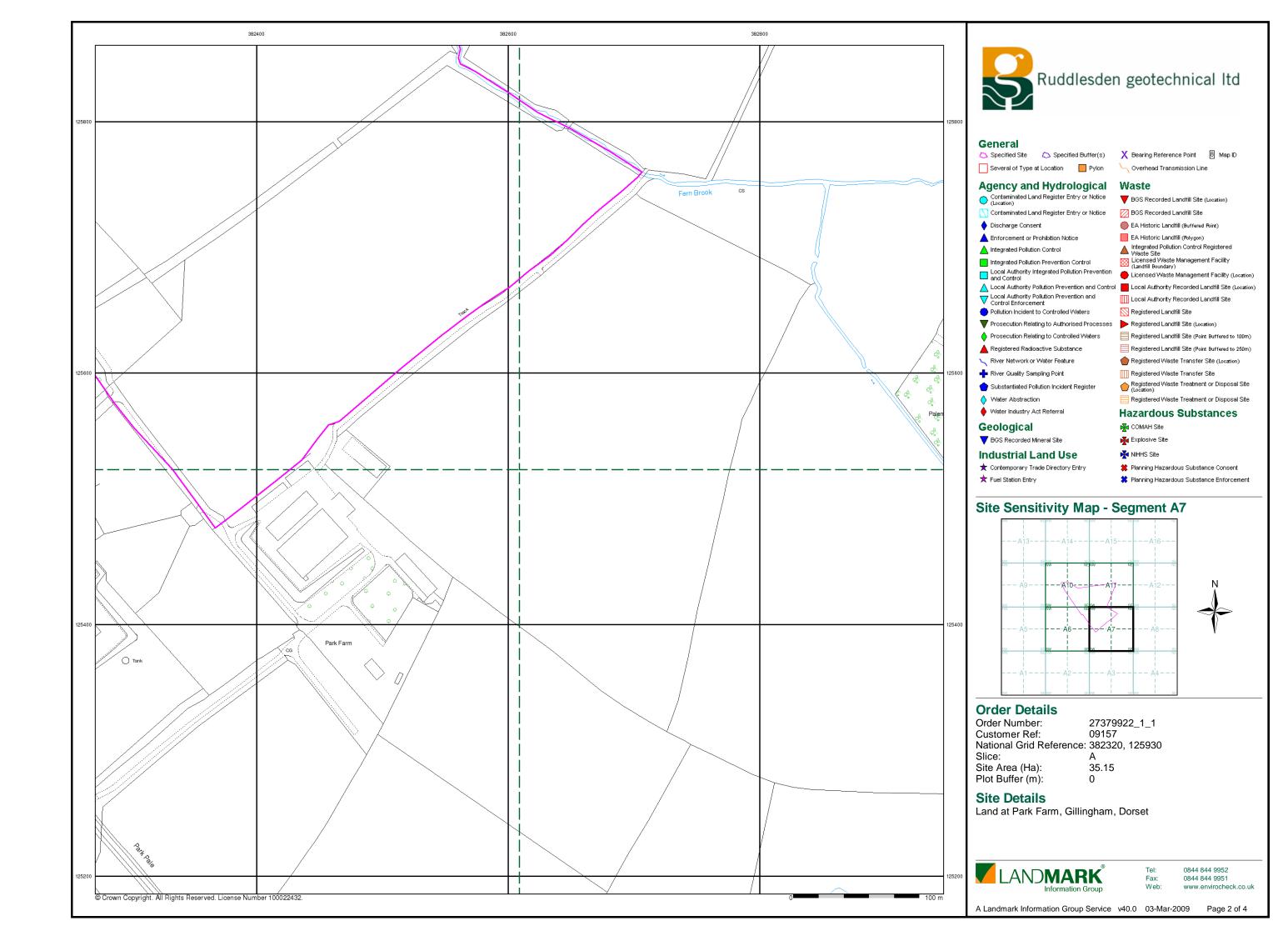
- 1. SGV = Soil Guideline Value
- 2. SGV (OLD) = Old Soil Guideline Value (used in the absence of a replacement)
- 3. LQM/CIEH = Land Quality Management/ Chartered Institute of Environmental Health
- 4. BRE = Building Research Establishment (Special Digest 1)
- 5. ICRCL = Inter-Departmental Committee on the Redevelopment of Contaminated Land
- 6. DUTCH = Dutch Value

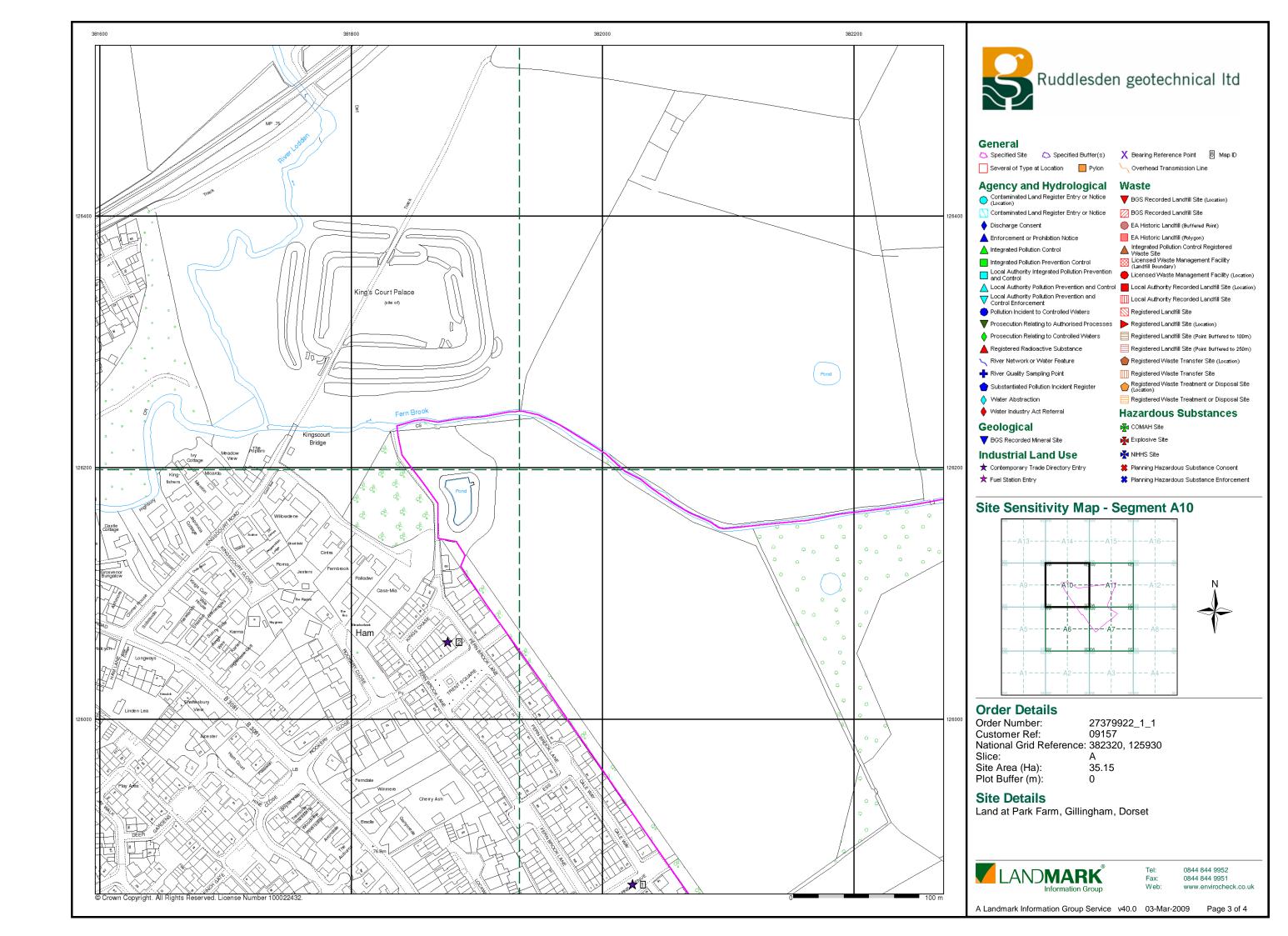


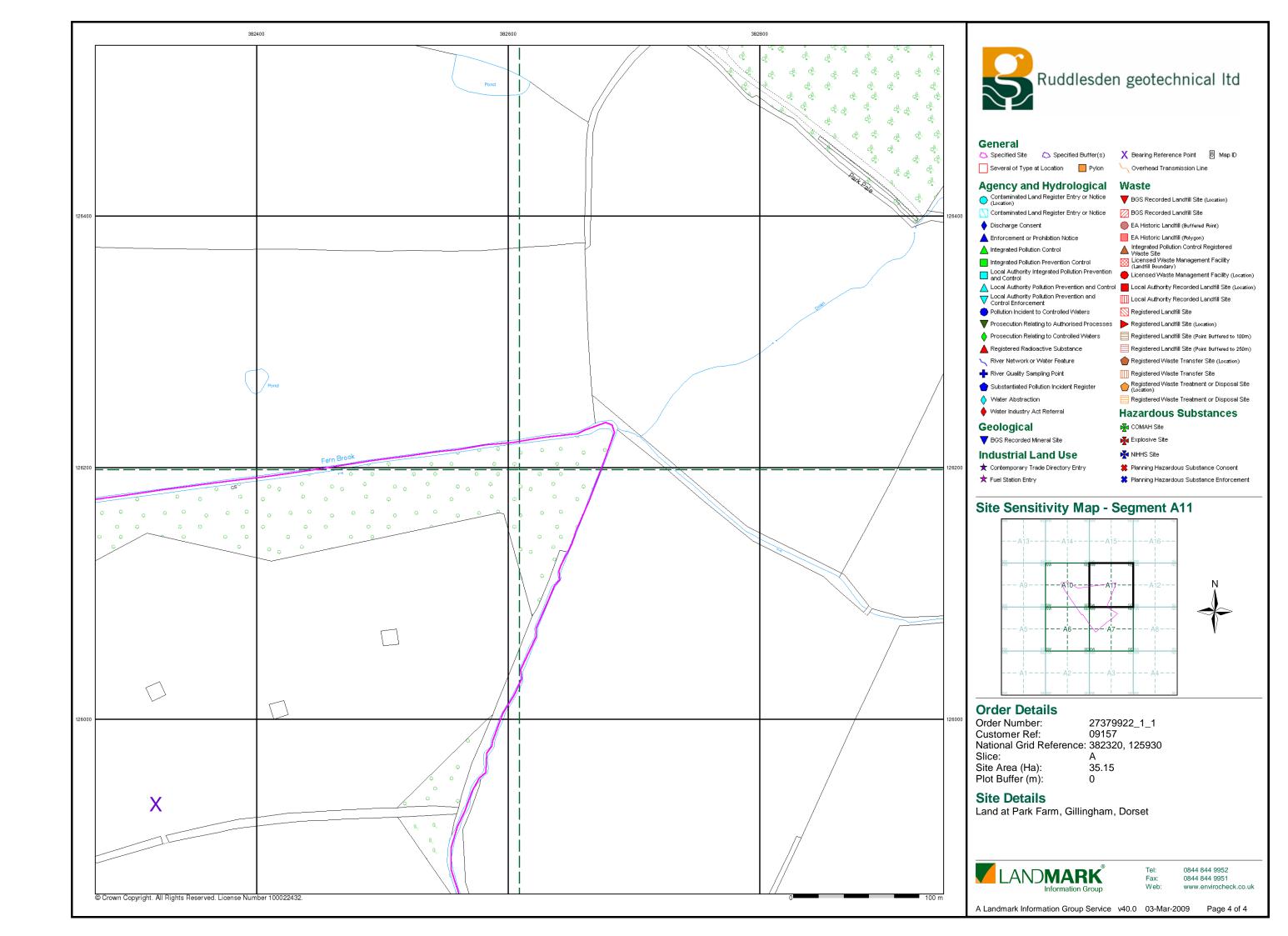
# APPENDIX D LANDMARK ENVIROCHECK REPORT

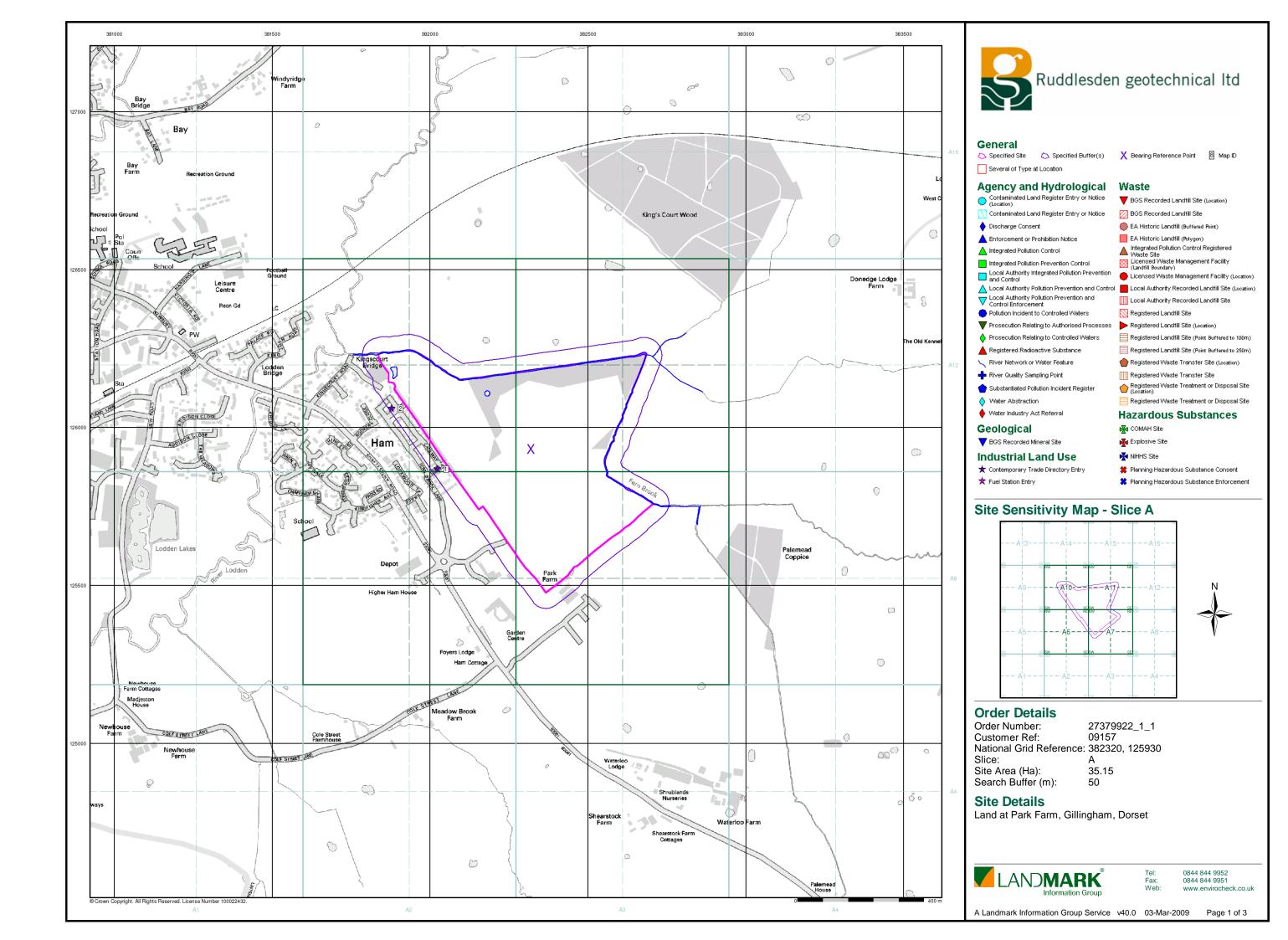


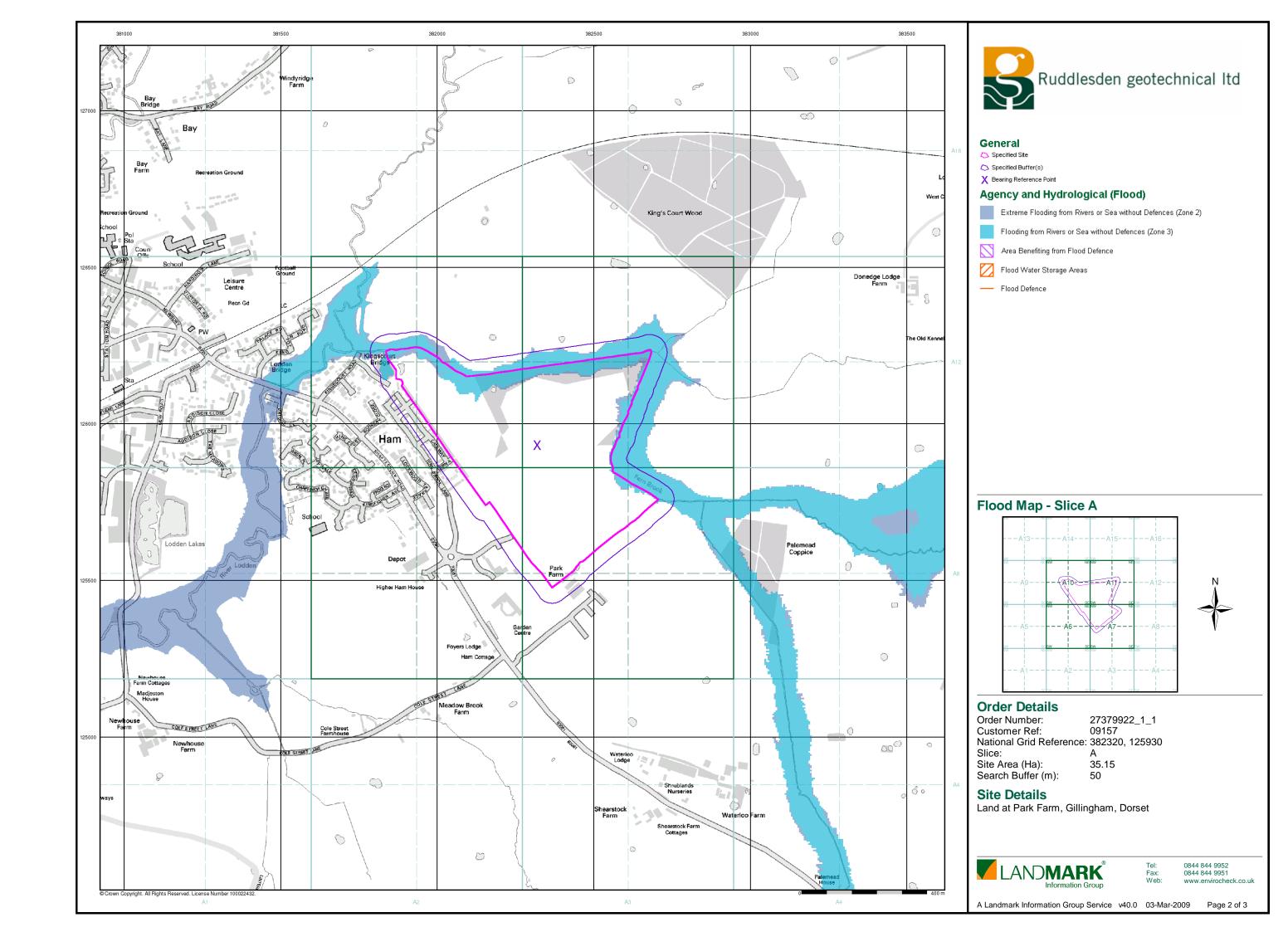


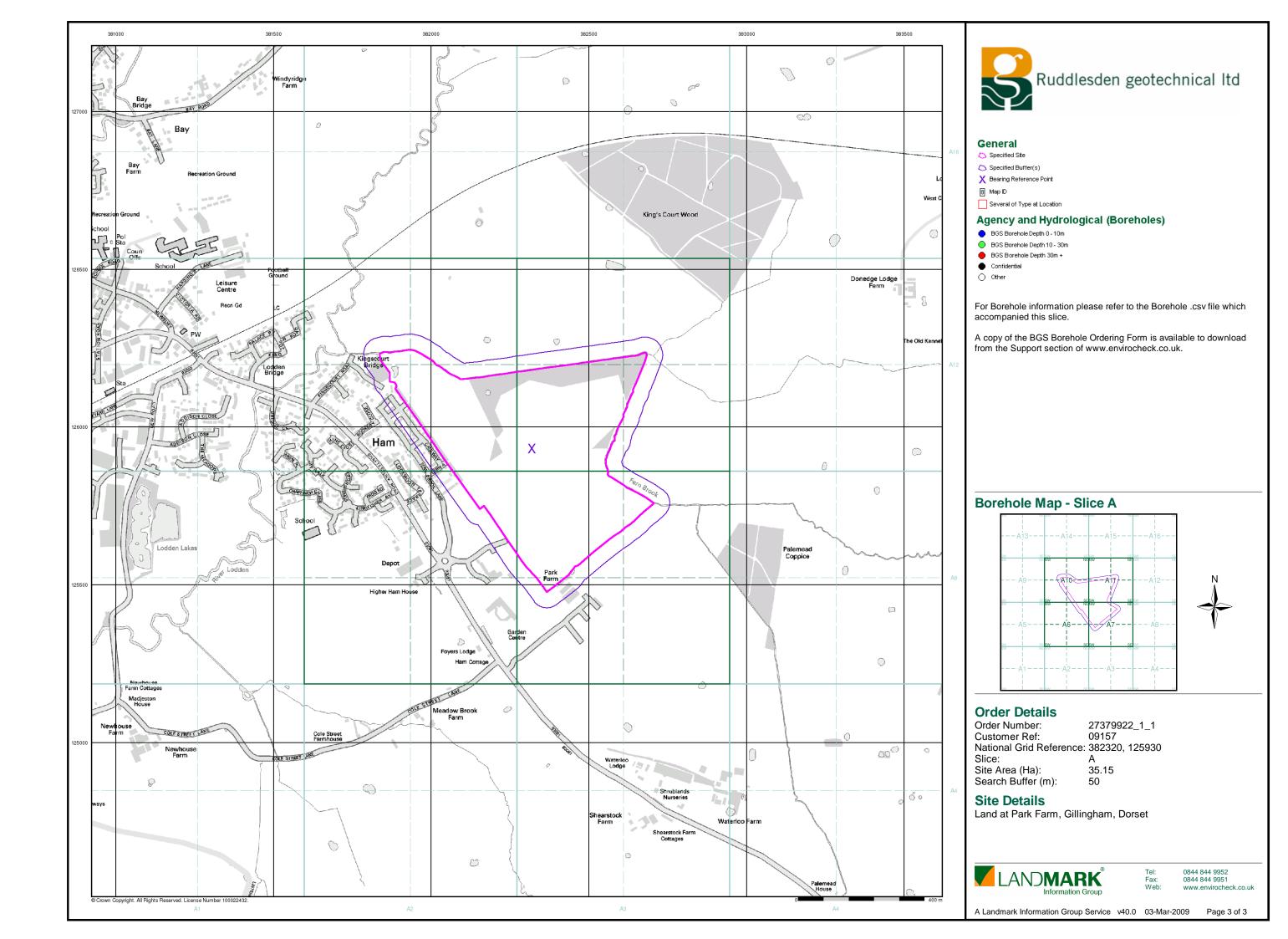












# **Historical Mapping Legends**

## **Ordnance Survey County Series 1:10,560** Gravel Pit Other Orchard Mixed Wood Deciduous Brushwood Furze Rough Pasture Arrow denotes Trigonometrical flow of water Station Site of Antiquities Bench Mark Pump, Guide Post, Well, Spring, Signal Post **Boundary Post** · 285 Surface Level Sketched Instrumental Contour Contour Fenced Fenced Main Roads Minor Roads Un-Fenced Raised Road Sunken Road Railway over Road over Railway Ri∨er Railway over Level Crossing Road Road over Road over

Road over

Co. Boro. Bdy.

Co. Burgh Bdy.

R.D. Bdy.

County Boundary (Geographical)

County & Civil Parish Boundary

County Borough Boundary (England)

County Burgh Boundary (Scotland)

Rural District Boundary

····· Civil Parish Boundary

Administrative County & Civil Parish Boundary

# Ordnance Survey Plan 1:10,000

Erran	Chalk Pit, Clay Pi	it	Gravel Pit
	Sand Pit		Disused Pit or Quarry
1.0.0.0	Refuse or Slag Heap		Lake, Loch or Pond
	Dunes	0000	Boulders
<b>* *</b> :	Coniferous Trees	$\triangle_{\Diamond}$	Non-Coniferous Trees
<b>ቀ</b> ቀ	Orchard Ωn_	Scrub	∖Y₁v Coppice
ជា ជា	Bracken	Heath ''	ı,,, Rough Grassland
<u> </u>	− Marsh 、、、V///	Reeds -	<u> 노</u> 소스 Saltings
	Dire Building	ection of Flow of W	Shingle
	Glasshouse	Pylon	Sand
********	Sloping Masonry		Electricity Transmission Line
	g Embankı	ment 	Standard Gauge Multiple Track
Road ' Under		vel Foot ssing Bridge	Standard Gauge Single Track Siding, Tramway or Mineral Line
	Geographical C	ounty	
	— — Administrative or County of Ci	County, County Bo ty	rough
	Municipal Boro Burgh or Distric	ugh, Urban or Rura ct Council	d District,
		h or County Consti not coincident with ot	
	Civil Parish Shown alternately	when coincidence of l	boundaries occurs
BP, BS	Boundary Post or Stone	Pol Sta Po	olice Station
Ch	Church	PO Po	st Office
CH	Club House	PC Pu	ıblic Convenience
F E Sta	Fire Engine Station		ıblic House
FB	Foot Bridge	SB Si	gnal Box
Fn	Fountain	Spr Sp	oring
CD	Outstand Desert	TOD T	Jambana Call Day

TCB

TCP

**Guide Post** 

Mile Post

Telephone Call Box

Telephone Call Post

## 1:10,000 Raster Mapping

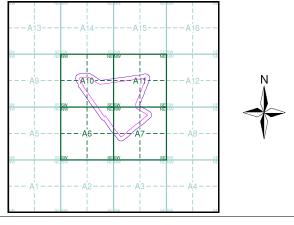
	Gravel Pit		Refuse tip or slag heap
	Rock	3 3	Rock (scattered)
	Boulders		Boulders (scattered)
	Shingle	Mud	Mud
Sand	Sand		Sand Pit
**********	Slopes		Top of cliff
	General detail		Underground detail
	Overhead detail		Narrow gauge railway
	Multi-track railway		Single track railway
-•-•	County boundary (England only)	• • • • •	Ci∨il, parish or community boundary
	District, Unitary, Metropolitan, London Borough boundary		Constituency boundary
۵ <sup>۵</sup>	Area of wooded vegetation	۵ <sup>۵</sup>	Non-coniferous trees
$\Box$	Non-coniferous trees (scattered)	** **	Coniferous trees
*	Coniferous trees (scattered)	Ċ̈́	Positioned tree
4 4 4 4	Orchard	* *	Coppice or Osiers
affr,	Rough Grassland	www.	Heath
On_ On_	Scrub	7/√\r 7/√\r	Marsh, Salt Marsh or Reeds
5	Water feature	<b>←</b>	Flow arrows
MHW(S)	Mean high water (springs)	MLW(S)	Mean low water (springs)
	Telephone line (where shown)	<b></b>	Electricity transmission line (with poles)
← BM 123.45 m	Bench mark (where shown)	Δ	Triangulation station
	Point feature (e.g. Guide Post or Mile Stone)	$\boxtimes$	Pylon, flare stack or lighting tower
•‡•	Site of (antiquity)		Glasshouse
	General Building		Important Building



# **Historical Mapping & Photography included:**

Mapping Type	Scale	Date	Pg
Dorset	1:10,560	1886	2
Wiltshire	1:10,560	1890	3
Dorset	1:10,560	1902	4
Dorset	1:10,560	1930 - 1931	5
Dorset	1:10,560	1938	6
Historical Aerial Photography	1:10,560	1945 - 1950	7
Ordnance Survey Plan	1:10,000	1962	8
Ordnance Survey Plan	1:10,000	1968	9
Ordnance Survey Plan	1:10,000	1985 - 1988	10
Ordnance Survey Plan	1:10,000	1993	11
10K Raster Mapping	1:10,000	2000	12
10K Raster Mapping	1:10,000	2008	13

# **Historical Map - Slice A**



#### **Order Details**

Order Number: 27379922\_1\_1
Customer Ref: 09157
National Grid Reference: 382320, 125930

Slice:

Site Area (Ha): 35.15 Search Buffer (m): 50

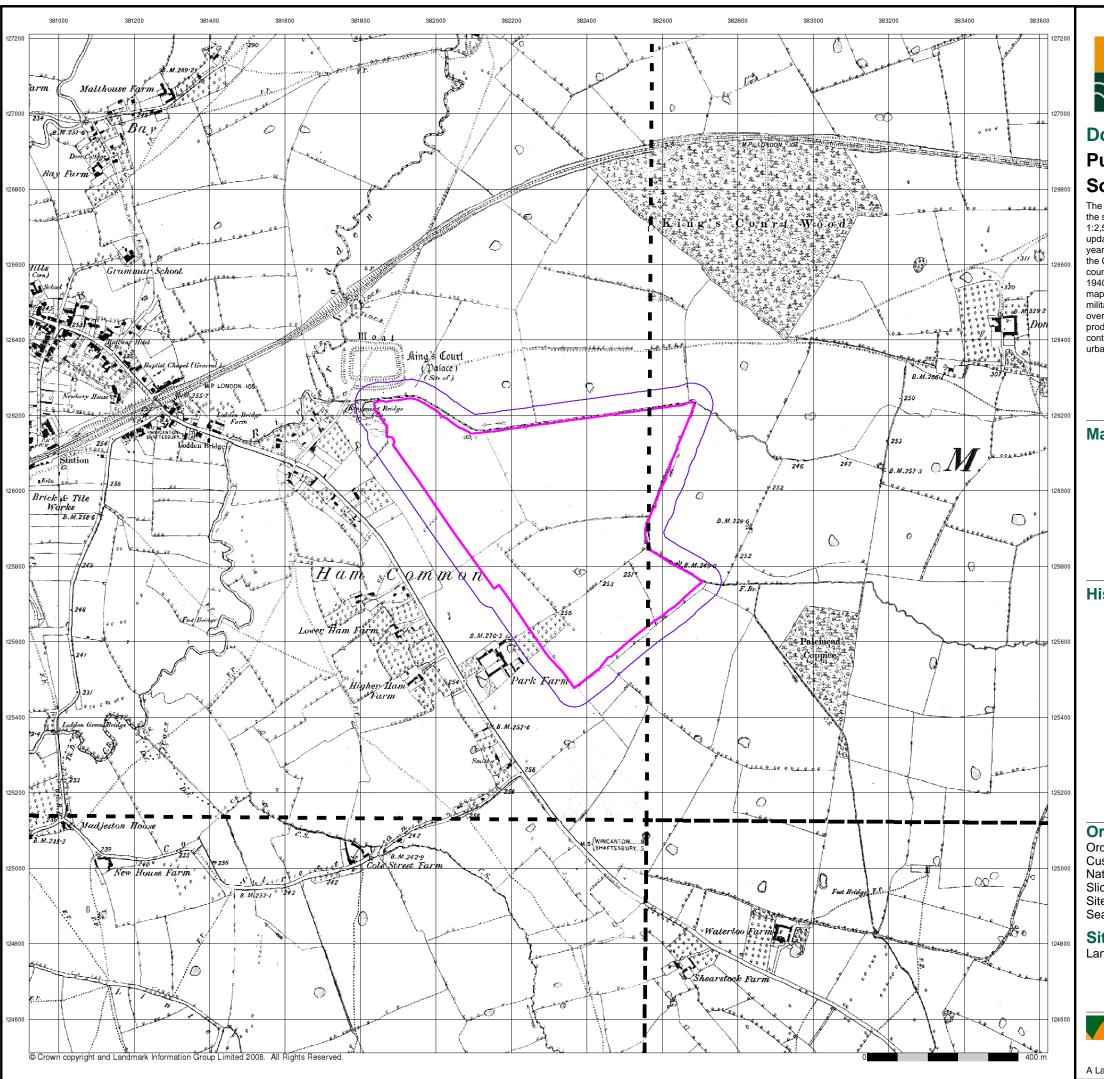
#### **Site Details**

Land at Park Farm, Gillingham, Dorset



Tel: 0844 844 9952 Fax: 0844 844 9951 Web: www.enviroched

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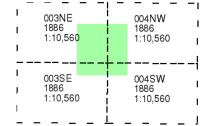




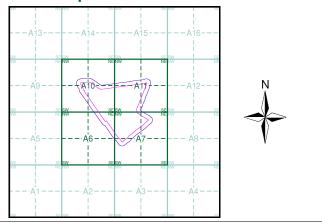
# Published 1886 Source map scale - 1:10,560

The historical maps shown were reproduced from maps predominantly held at the scale adopted for England, Wales and Scotland in the 1840's. In 1854 the 1:2,500 scale was adopted for mapping urban areas; these maps were used to update the 1:10,560 maps. The published date given therefore is often some years later than the surveyed date. Before 1938, all OS maps were based on the Cassini Projection, with independent surveys of a single county or group of counties, giving rise to significant inaccuracies in outlying areas. In the late 1940's, a Provisional Edition was produced, which updated the 1:10,560 mapping from a number of sources. The maps appear unfinished - with all military camps and other strategic sites removed. These maps were initially overprinted with the National Grid. In 1970, the first 1:10,000 maps were produced using the Transverse Mercator Projection. The revision process continued until recently, with new editions appearing every 10 years or so for urban areas.

# Map Name(s) and Date(s)



# **Historical Map - Slice A**



#### **Order Details**

Order Number: 27379922\_1\_1
Customer Ref: 09157
National Grid Reference: 382320, 125930

Slice:

Site Area (Ha): 35.15 Search Buffer (m): 50

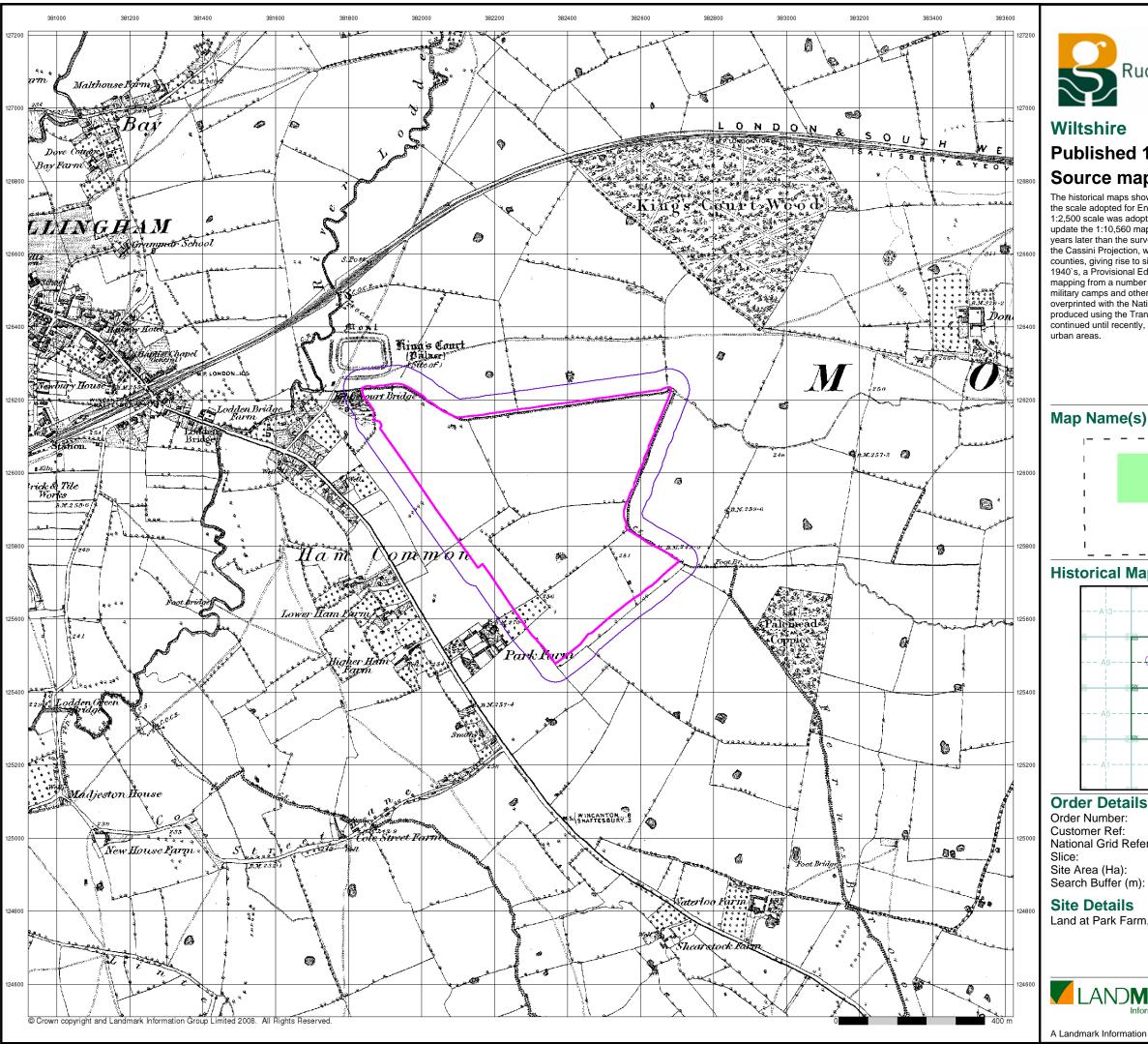
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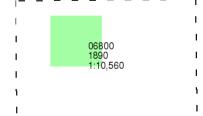


## Wiltshire

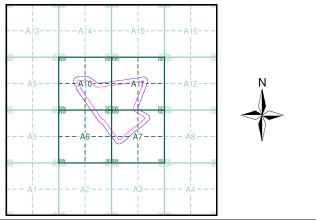
# **Published 1890** Source map scale - 1:10,560

The historical maps shown were reproduced from maps predominantly held at the scale adopted for England, Wales and Scotland in the 1840's. In 1854 the 1:2,500 scale was adopted for mapping urban areas; these maps were used to update the 1:10,560 maps. The published date given therefore is often some years later than the surveyed date. Before 1938, all OS maps were based on the Cassini Projection, with independent surveys of a single county or group of counties, giving rise to significant inaccuracies in outlying areas. In the late 1940's, a Provisional Edition was produced, which updated the 1:10,560 mapping from a number of sources. The maps appear unfinished - with all military camps and other strategic sites removed. These maps were initially overprinted with the National Grid. In 1970, the first 1:10,000 maps were produced using the Transverse Mercator Projection. The revision process continued until recently, with new editions appearing every 10 years or so for urban areas.

# Map Name(s) and Date(s)



#### **Historical Map - Slice A**



#### **Order Details**

Order Number: 27379922\_1\_1 Customer Ref: 09157 National Grid Reference: 382320, 125930

35.15 Site Area (Ha):

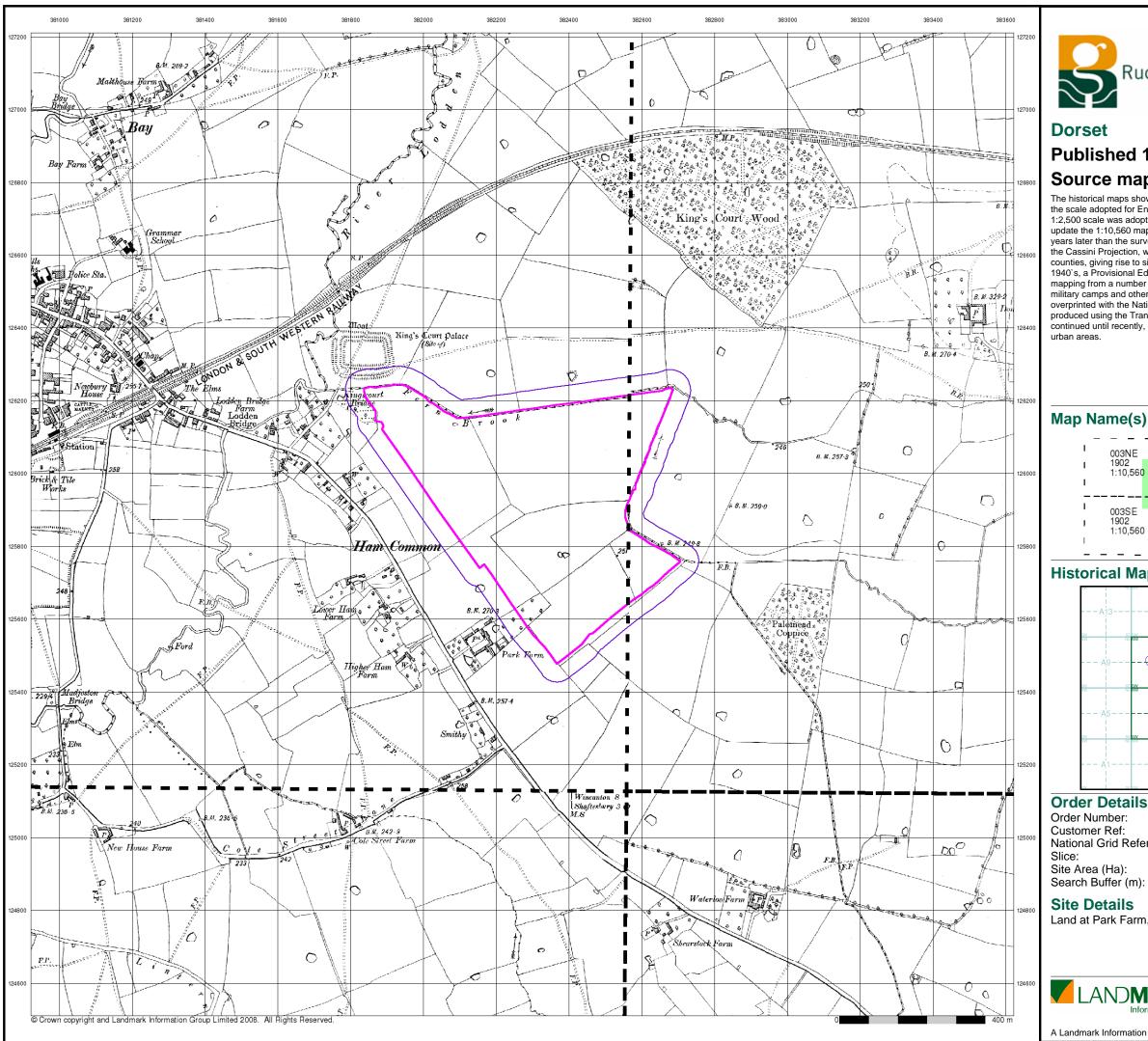
**Site Details** 

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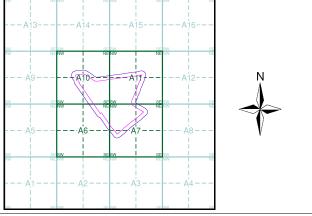
# Published 1902 Source map scale - 1:10,560

The historical maps shown were reproduced from maps predominantly held at the scale adopted for England, Wales and Scotland in the 1840's. In 1854 the 1:2,500 scale was adopted for mapping urban areas; these maps were used to update the 1:10,560 maps. The published date given therefore is often some years later than the surveyed date. Before 1938, all OS maps were based on the Cassini Projection, with independent surveys of a single county or group of counties, giving rise to significant inaccuracies in outlying areas. In the late 1940's, a Provisional Edition was produced, which updated the 1:10,560 mapping from a number of sources. The maps appear unfinished - with all military camps and other strategic sites removed. These maps were initially overprinted with the National Grid. In 1970, the first 1:10,000 maps were produced using the Transverse Mercator Projection. The revision process continued until recently, with new editions appearing every 10 years or so for urban areas.

## Map Name(s) and Date(s)

I	003NE	004NW	
I	1902	1902	
I	1:10,560	1:10,560	
 	003SE 1902 1:10,560	004SW 1902 1:10,560	

# **Historical Map - Slice A**



#### **Order Details**

Order Number: 27379922\_1\_1 Customer Ref: 09157

National Grid Reference: 382320, 125930

Slice: Site Area (Ha): 35.15

50

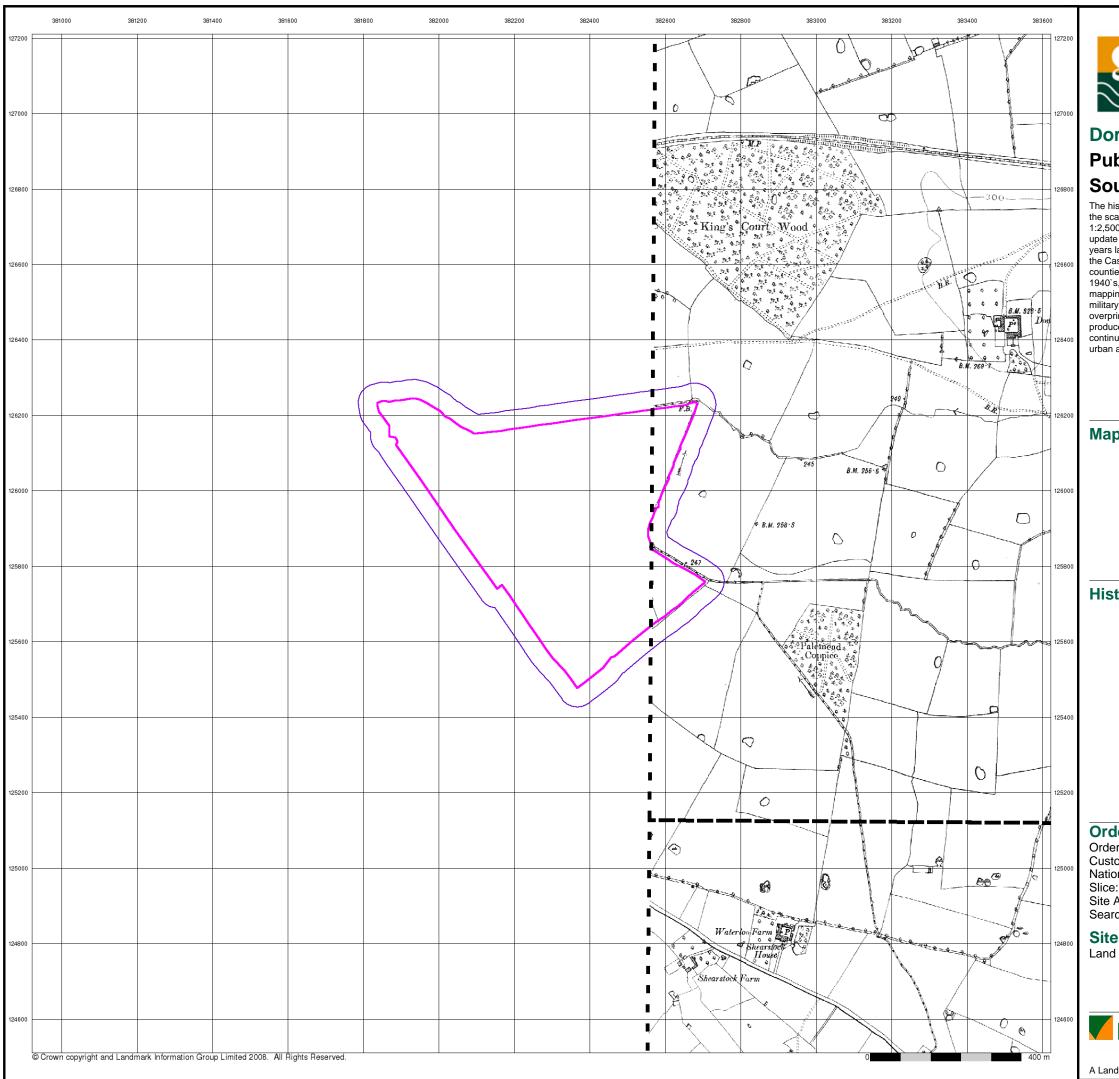
**Site Details** 

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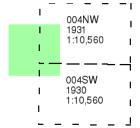




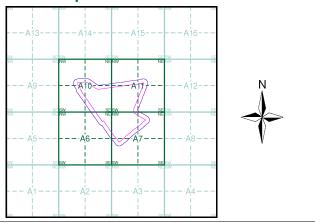
# **Published 1930 - 1931** Source map scale - 1:10,560

The historical maps shown were reproduced from maps predominantly held at the scale adopted for England, Wales and Scotland in the 1840's. In 1854 the 1:2,500 scale was adopted for mapping urban areas; these maps were used to update the 1:10,560 maps. The published date given therefore is often some years later than the surveyed date. Before 1938, all OS maps were based on the Cassini Projection, with independent surveys of a single county or group of counties, giving rise to significant inaccuracies in outlying areas. In the late 1940's, a Provisional Edition was produced, which updated the 1:10,560 mapping from a number of sources. The maps appear unfinished - with all military camps and other strategic sites removed. These maps were initially overprinted with the National Grid. In 1970, the first 1:10,000 maps were produced using the Transverse Mercator Projection. The revision process continued until recently, with new editions appearing every 10 years or so for urban areas.

# Map Name(s) and Date(s)



#### **Historical Map - Slice A**



#### **Order Details**

Order Number: 27379922\_1\_1 Customer Ref: 09157

National Grid Reference: 382320, 125930

35.15 Site Area (Ha): Search Buffer (m): 50

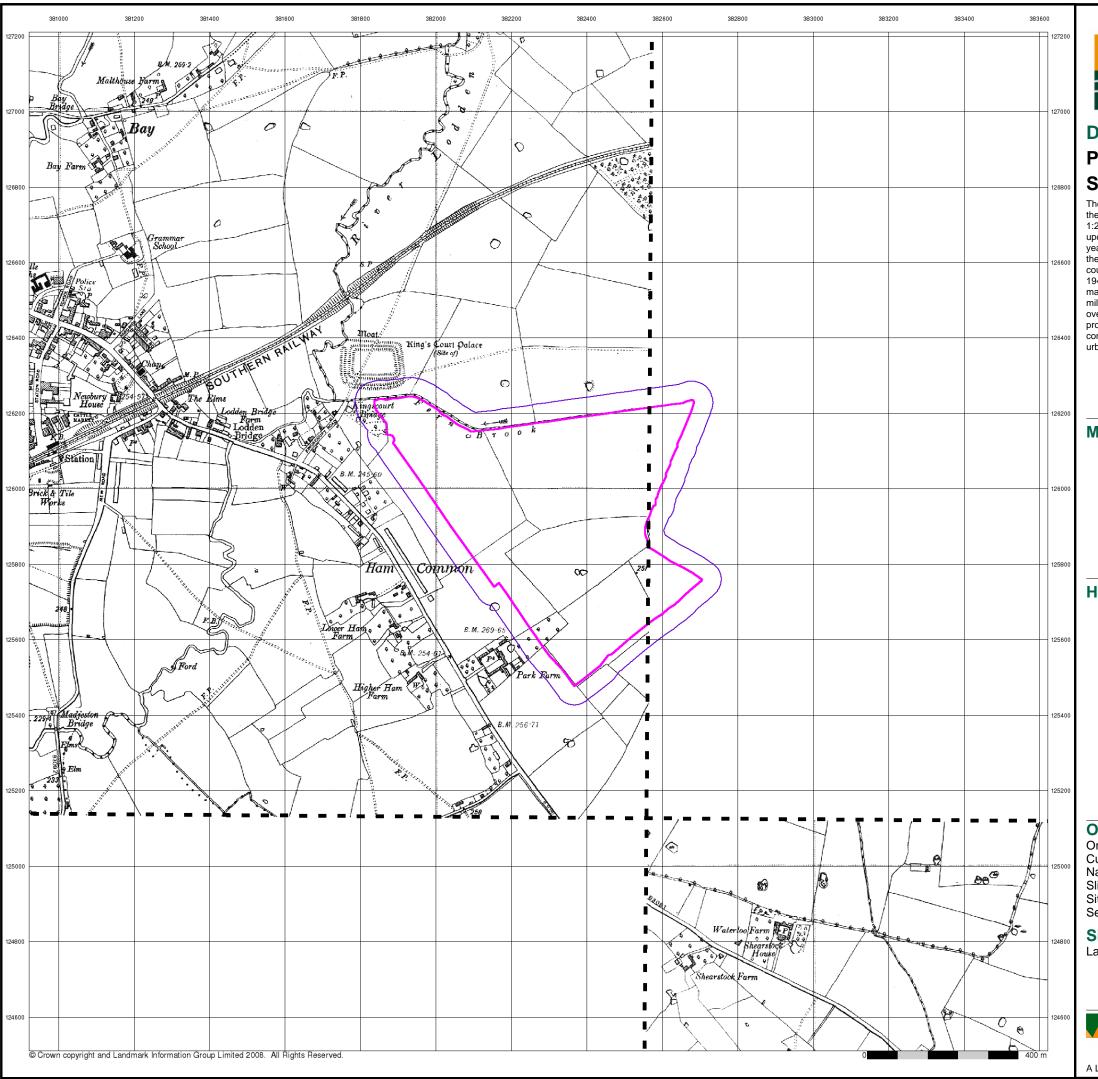
#### **Site Details**

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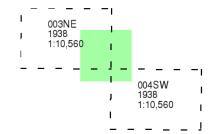




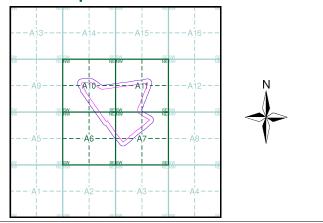
# Published 1938 Source map scale - 1:10,560

The historical maps shown were reproduced from maps predominantly held at the scale adopted for England, Wales and Scotland in the 1840's. In 1854 the 1:2,500 scale was adopted for mapping urban areas; these maps were used to update the 1:10,560 maps. The published date given therefore is often some years later than the surveyed date. Before 1938, all OS maps were based on the Cassini Projection, with independent surveys of a single county or group of counties, giving rise to significant inaccuracies in outlying areas. In the late 1940's, a Provisional Edition was produced, which updated the 1:10,560 mapping from a number of sources. The maps appear unfinished - with all military camps and other strategic sites removed. These maps were initially overprinted with the National Grid. In 1970, the first 1:10,000 maps were produced using the Transverse Mercator Projection. The revision process continued until recently, with new editions appearing every 10 years or so for urban areas.

## Map Name(s) and Date(s)



#### **Historical Map - Slice A**



#### **Order Details**

Order Number: 27379922\_1\_1
Customer Ref: 09157
National Grid Reference: 382320, 125930

Slice:

Site Area (Ha): 35.15 Search Buffer (m): 50

#### **Site Details**

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